



Equilibrium Morphological Modelling in Coastal and River Environments: The Development and Application of Self-Organisation- and Entropy-Based Techniques.

By Joanna M. Nield

Submitted in fulfilment of the requirements for the degree of
DOCTOR OF PHILOSOPHY

December 2005

FACULTY OF ENGINEERING, COMPUTER AND MATHEMATICAL SCIENCES
School of Civil and Environmental Engineering

Equilibrium Morphological Modelling in Coastal and River Environments: The Development and Application of Self-Organisation- and Entropy-Based Techniques.

By Joanna M. Nield

Submitted for examination: DECEMBER 2005
Comments received: MARCH 2006
Amendments completed: JUNE 2006

Thesis submitted in fulfilment of the requirements for the degree of
Doctor of Philosophy

School of Civil and Environmental Engineering
Faculty of Engineering, Computer and Mathematical Sciences

The University of Adelaide SA 5005 Australia
Telephone: +61 8 8303 3744
Facsimile: +61 8 8303 4359
Web: www.civeng.adelaide.edu.au
Email: enquiries@civeng.adelaide.edu.au

Abstract

The planning and management of coastal and river structures such as breakwaters, groynes, jetties, bridges and tidal inlets require accurate predictions of equilibrium morphologies. Generally these types of situations are modelled numerically using process-based models, where wave, current and sediment transport modules are applied over a number of time-steps until a steady-state morphology is obtained. Two alternative methods have been developed and applied in this thesis, based on self-organisation and entropy approaches.

The self-organisation-based method utilises a cellular automata model, where local rules produce a global stable pattern through positive and negative feedback. The entropy-based method is able to predict equilibrium morphologies directly. It compares different randomly generated morphologies using an objective function and optimisation, instead of moving to an equilibrium morphology through intermediate states. This avoids some potential problems associated with traditional models such as error propagation and reliance on accurate initial conditions.

The models developed in this thesis have been applied to a number of case studies. It was found that the cellular automata model obtained a higher Brier Skill Score than a comparable process-based model when predicting the equilibrium morphology associated with a channel obstruction. The entropy-based method was able to predict a realistic erosional channel in a coastal lagoon, similar to field observations at the Murray River Mouth in South Australia. It had difficulties predicting the deposition pattern due to the bias of the objective function towards erosional environments. The entropy-based method outperformed a conventional model prediction of the equilibrium erosional channel associated with a laboratory-sized lagoon, but similar problems were observed with its deposition predictive ability.

The modelling methods developed in this thesis are a first step into the use of non-traditional, entropy- and self-organisation-based models for the prediction of complex equilibrium morphologies. They have made use of non-conventional models in order to explore different objective function formulations or self-organisation rules and the sensitivity of these, and have compared the models to laboratory results. The work documented in this dissertation shows that it is possible to use self-organisation- and entropy-based modelling methods to predict stable, equilibrium morphologies in coastal and river environments.

Statement of Originality

I Joanna M. Nield hereby declare that this work contains no material that has been accepted for the award of any other degree or diploma in any university or other tertiary institution. To the best of my knowledge and belief, it contains no material previously published or written by any other person, except where due reference is made in the text.

I give consent to this copy of my thesis, when deposited in the University Library, being available in all forms of media, now or hereafter known.

SIGNED: DATE:

Acknowledgements

I wish to thank the many people that have helped contribute to the production of this thesis through their help, advise, support and useful discussions:

- ★ My supervisors Dr David Walker and Assoc. Prof. Martin Lambert, without whom this project would never have made it to the form it is now in. Their ideas, discussions and suggestions have been invaluable.
- ★ My family (Melanie, Steven and Abigail Nield) and Steven Whichelo for all their support over the last 3.75 years. I greatly appreciate their help and perseverance. I also acknowledge the proofreading they have attempted of the Introduction and Conclusion chapters of this thesis, as well as reading my journal and conference papers and listening to practise presentations.
- ★ The role of the Australian Research Council in providing funding for this project and the Department of Water, Land and Biodiversity Conservation as the industry partner (project no. LP0227320).
- ★ Joe Davis for his help in the use of the laser scanner, for being a Matlab guru and never getting sick of me knocking on his door to ask a question.
- ★ The instrumentation and laboratory staff of the School of Civil and Environmental Engineering at the University of Adelaide, for their assistance in the laboratory studies.
- ★ Marcos Bernardes from the University of Plymouth for his help with the Brier Skill Score.
- ★ Researchers at the University of Plymouth and Universitat Politècnica de Catalunya for useful meetings and discussions.
- ★ The organisers, lecturers and participants at the Renesse course, for giving me a strong base, endless list of contacts and many fond memories.
- ★ Everyone that I have met on conferences that have helped me form ideas during lively discussions.

Contents

Abstract	i
Statement of Originality	iii
Acknowledgements	v
List of Figures	x
List of Tables	xviii
List of Symbols	xix
List of Abbreviations	xxi
1 Introduction	1
1.1 Equilibrium Morphology	1
1.1.1 Traditional Modelling Methods	3
1.2 Aims	4
1.2.1 Development of New Modelling Methods	4
1.2.2 Case Studies	5
1.2.3 Differences Between the New and Traditional Modelling Methods	7
1.3 Research Outline and Approach	7
2 Literature Review	11
2.1 General Modelling Methods Currently Used For Medium- and Long-Term Predictions	11
2.1.1 Process-Based Models and Their Potential Limitations	15
2.2 Use of Entropy-Based Equilibrium Modelling in River Morphology	18
2.2.1 Open, Closed and Isolated System Definitions and Entropy Behaviour	19
2.2.2 Leopold and Langbein's Analogy	21
2.2.3 Extrema Hypotheses and Their Use in Fluvial Systems	23
2.2.4 Minimum Energy Dissipation Rate	24
2.2.5 The Use of Other Entropy-Based Analyses in Coastal Environments	27
2.3 Optimisation: A Comparison of Genetic Algorithms and Simulated Annealing	28
2.3.1 The Use of GAs and SA in Morphological Areas	31
2.4 Self-organisation: Its Use in River and Coastal Modelling	32
2.5 The Research Gap	37
3 Self-Organisation-Based Method and its Application to River Channels and Lagoons	39
3.1 Method Description	40
3.2 Channel Constriction Case Study: Laboratory Method and Results	44
3.2.1 Flow and Depth Measurements	46
3.2.2 Velocity Measurements	46
3.2.3 Bathymetry Measurements	47
3.2.4 Channel Constriction Results.....	48
3.3 Channel Constriction Case Study: Traditional Process-Based Model Description and Results	53
3.4 Channel Constriction Case Study: Cellular Automata Results, Comparisons and Discussion	54
3.4.1 Comparison of Traditional and Self-Organisation-Based Model Results	55
3.4.2 Comparison of Self-Organisation-Based Model and Laboratory Results	56
3.4.3 Sensitivity Analyses: Use of Random Starting Morphology	56
3.4.4 Sensitivity Analyses: Grid Size	57

3.4.5	Sensitivity Analyses: Fixed or Variable Avalanche Criterion	57
3.5	Channel Obstruction Case Study: Laboratory Method and Results	58
3.6	Brier Skill Score Definition	62
3.7	Channel Obstruction Case Study: Traditional Process-Based Model Description and Results.....	64
3.8	Channel Obstruction Case Study: Cellular Automata Results, Comparisons and Discussion	65
3.8.1	Sensitivity Analyses: Random Starting Morphologies	67
3.8.2	Sensitivity Analyses: Different Sediment Increment Sizes	68
3.8.3	Sensitivity Analyses: Different Grid Sizes	68
3.8.4	Sensitivity Analyses: Fixed or Variable Angle of Repose.....	69
3.8.5	Sensitivity Analyses: Angle of Repose Value	70
3.8.6	Sensitivity Analyses: Sediment Grain Size.....	71
3.8.7	Rule Violations Pattern	71
3.8.8	Comparison of Laboratory Velocities and Hydrodynamic Model Calculations	72
3.8.9	Conclusions.....	76
3.9	Unidirectional Lagoon Example	77
3.9.1	Case Study Definition	77
3.9.2	Results and Discussion	78
3.9.3	Energy Dissipation Trends – A Link to Entropy-Based Modelling.....	79
4	Entropy-Based Method and its Application to Field and Laboratory Lagoons.....	81
4.1	Method Description.....	82
4.2	Global Search Method Characterisation	86
4.2.1	Method	87
4.2.2	Real Genetic Algorithm Description.....	89
4.2.3	Binary Genetic Algorithm Description	92
4.2.4	Binary and Real Genetic Algorithm Comparison	93
4.2.5	Simulated Annealing Description	94
4.2.6	Real Genetic Algorithm and Simulated Annealing Comparison	96
4.2.7	Combination Comparison	99
4.2.8	Optimisation Conclusions	102
4.3	Unidirectional Lagoon Modelling.....	102
4.3.1	Optimisation Parameters Utilised	102
4.3.2	Results of General Application	104
4.3.3	Sequential SAs	105
4.3.4	Sensitivity Analyses	106
4.3.5	Discussion of Unidirectional Lagoon Results.....	111
4.4	Expansion to Reversing Flow Lagoon Modelling	113
4.4.1	Method	113
4.4.2	Results.....	114
4.5	Application to a Lagoon Laboratory Experiment	118
4.5.1	Laboratory Experiment	119
4.5.2	Traditional Modelling Results	121
4.5.3	Modelling Modifications.....	122
4.5.4	Modelling Results and Laboratory Comparison	123
4.5.5	Discussion and Conclusions	125
4.6	Summary	126

5	Application to Coastal Situations – Detached Breakwater.....	129
5.1	Introduction.....	129
5.2	Model Description	131
5.3	Case Study Description.....	136
5.4	Breakwater Model Results and Sensitivity Analyses	138
5.4.1	Sensitivity to Initial Random Morphologies.....	141
5.4.2	Inclusion of Variable Limits with Water Depth	143
5.4.3	Coarse and Fine Fixed Grid Size Comparisons	147
5.4.4	Use of Different Fixed Limits with a Coarse Morphological Grid.....	151
5.4.5	Inclusion of Minimum Elevation Variance in Objective Function.....	153
5.5	Discussion.....	157
5.6	Conclusions.....	159
6	Conclusions and Recommendations.....	161
6.1	Conclusions.....	161
6.1.1	Self-Organisation-Based Method	161
6.1.2	Entropy-Based Method	163
6.1.3	Application of Optimisation Method to a Coastal Breakwater.....	165
6.2	Recommendations for Further Research.....	167
6.2.1	Self-Organisation-Based Model	167
6.2.2	Entropy-Based Model	168
6.2.3	Breakwater Application	168
6.3	Closing Remark	169
7	References	171
8	Publications.....	179
	Appendix A - Self-Organisation Model Code.....	181
	Appendix B - Comparison of Optimisation Methods	203
	Appendix C - Optimisation Model Code	205
	Appendix D - Laboratory Lagoon Experiment Results	305
	Appendix E - Laboratory Breakwater Experiment Results	311

List of Figures

Figure 1.1	Examples of equilibrium morphologies that form in the lee of breakwaters. (a) Semaphore Beach, Adelaide, Australia, (b) Costa Brava, Spain, (c) Blanes, Spain.....	1
Figure 1.2	Examples of equilibrium morphologies that form due to groyne placement. (a) Somerton Beach, Adelaide, Australia, (b) Herne Bay Beach, Kent, UK, (c) Glenelg Beach, Adelaide, Australia.	2
Figure 1.3	Examples of equilibrium morphologies associated with different river flow and tidal conditions through a tidal inlet at the Murray Mouth, South Australia during 1980 and 2000.....	2
Figure 1.4	Example from Sloff et al. (2004) of slightly different initial conditions producing a completely different modelling result, with a channel forming on the opposite side of the flume to that observed in laboratory experiments.	4
Figure 1.5	Examples of the (a) constriction and (b) obstruction laboratory case studies to which the self-organisation based model was applied.	6
Figure 1.6	Examples of the unidirectional sandy lagoon of (a) field sized and (b) laboratory sized case studies to which the self-organisation-based and entropy-based models were applied.	6
Figure 2.1	General conventional process-based coastal model structure for predicting morphologies at equilibrium.	12
Figure 2.2	Laboratory results obtained in a wide reservoir study (after Sloff et al., 2004).	16
Figure 2.3	Typical longitudinal equilibrium river profile determined by using entropy-based principles (after Fiorentino and Claps, 1992).	19
Figure 2.4	Definition of Open, Closed and Isolated Systems.....	20
Figure 2.5	Optimal channel network (after Rinaldo, 1999).....	25
Figure 2.6	An example of five river delta channels analysed by Wright et al. (after Wright et al., 1973)	28
Figure 3.1	Description of modelled case studies (a) a channel constriction, (b) a channel obstruction and (c) a unidirectional lagoon.	40
Figure 3.2	General description of model methodology.	41
Figure 3.3	Von Neumann neighbourhood.....	42
Figure 3.4	Constriction case study description.	44
Figure 3.5	Constriction laboratory set-up.	45
Figure 3.6	(a) The top moving mechanism of the scanner, (b) and (c), the laser measuring part of the scanner, measurements were taken while the flume bed was still submerged.	47
Figure 3.7	Contour plot of a typical channel bed. The contours are plotted at 25mm intervals. The flow was from left to right.	48
Figure 3.8	Contour plot of (a) Experiment Two results and (b) Experiment Three results.	49
Figure 3.9	Comparison of Experiment One (black) and Four (white). Contours are at 25mm intervals.....	49

Figure 3.10 Contour plot of (a) Experiment Four results and (b) Experiment Five results.	50
Figure 3.11 Variation of depth to bottom and point velocity (averaged over a one minute interval) with time for Experiment Five.	50
Figure 3.12 Variation of depth to bottom and average velocity across the flume for Experiment Six.	51
Figure 3.13 Variation of depth to bottom and average velocity with time for Experiment Six.	52
Figure 3.14 Contour plot (at 50mm intervals) of traditional model once simulation had reached equilibrium position. Flow was from left to right.	54
Figure 3.15 Contour plot (at 50mm intervals) of a sample bed morphology resulting from the self-organisation model. Flow was from left to right.	55
Figure 3.16 Longitudinal cross-section of the channel morphology – a comparison of the process-based and self-organisation-based methods. Flow direction was from left to right.	55
Figure 3.17 Longitudinal cross-section of the channel morphology – a comparison of the self-organisation-based method and laboratory results.	56
Figure 3.18 Comparison of longitudinal cross-sections from two different random starting configurations. Flow direction was from left to right.	57
Figure 3.19 Comparison between coarse and fine grid model results through centre of constriction. Flow direction was from left to right.	57
Figure 3.20 Comparison of model runs with different avalanche mechanisms. Flow direction was from left to right.	58
Figure 3.21 Obstruction case study description.	58
Figure 3.22 Obstruction laboratory set-up.	59
Figure 3.23 The obstruction in the rectangular flume, before and during experiments.	59
Figure 3.24 Depth to bottom and velocity measurements from Experiment One, each point represents an average of values recorded over a one minute time period, measured using an ADV, positioned to the right of the obstruction (facing down flow).	60
Figure 3.25 Velocity profiles at equilibrium for Experiment One.	61
Figure 3.26 The resultant morphology in the channel from Experiment One, looking upstream and downstream respectively. The water and upper parts of the obstruction have been removed.	61
Figure 3.27 Illustration of a negative skill model result, in relation to a baseline measurement.	63
Figure 3.28 Morphology of the channel bed surrounding an obstruction obtained from (a) the traditional model, and (b) laboratory Experiment One. Flow was from left to right. Contour lines are at 25mm intervals.	65
Figure 3.29 Morphology of the channel bed surrounding an obstruction obtained from (a) the self-organisation model, and (b) laboratory Experiment One. Flow was from left to right. Contour lines are at 25mm intervals.	66

Figure 3.30 Longitudinal cross-section (A-A') comparison of channel morphology obtained from laboratory Experiment Two and the cellular automaton model. Flow was from left to right.....	66
Figure 3.31 Comparisons of channel morphology obtained from different starting morphologies using the self-organisation-based model. Flow was from left to right. Contour lines are at 25mm intervals.	67
Figure 3.32 Morphology of the channel bed surrounding an obstruction obtained from the self-organisation-based model using two different increment sizes of (a) 10mm and (b) 0.5mm. Flow was from left to right. Contour lines are at 25mm intervals.	68
Figure 3.33 Morphology of the channel bed surrounding an obstruction obtained from the self-organisation model using two different grid sizes of (a) 0.1 m and (b) 0.05 m. Flow was from left to right. Contour lines are at 25mm intervals.	69
Figure 3.34 Comparison of laboratory Experiment One and self-organisation model with fixed avalanche angle along the right side of the channel facing downstream.....	69
Figure 3.35 Longitudinal cross-section comparison of morphology on the right side of the obstruction (facing flow direction) obtained from the laboratory study and self-organisation-based modelling study using variable avalanche angle.	70
Figure 3.36 Contour plot of different angles of repose (40° (a) and 30° (b)). The contours are plotted at 25mm intervals. The flow was from left to right.	70
Figure 3.37 Contour plot of different d_{50} sizes ($140\mu\text{m}$ (a), $200\mu\text{m}$ (b), $280\mu\text{m}$ (c) and $340\mu\text{m}$ (d)). The contours are plotted at 25mm intervals. The flow was from left to right.	71
Figure 3.38 Path of self-organisation model for Experiment One set-up. Number of cells in which sediment increments are moved in each iteration and number of cells where avalanching occurs.....	72
Figure 3.39 Comparison of depth averaged velocity field obtained from model morphology and that measured near the bed in laboratory Experiment One, upstream of the obstruction.	73
Figure 3.40 Comparison of depth averaged velocity field obtained from model morphology and that measured near the bed in laboratory Experiment One, immediately downstream of the obstruction.....	74
Figure 3.41 Comparison of Velocity field obtained from model morphology and that measured in laboratory Experiment One, down stream of the obstruction and deposition mounds.	74
Figure 3.42 Comparison of depth averaged velocity field obtained from model morphology and that measured near the bed in laboratory Experiment Two, upstream of the obstruction.	75
Figure 3.43 Comparison of depth averaged velocity field obtained from model morphology and that measured near the bed in laboratory Experiment Two, immediately downstream of the obstruction.....	75
Figure 3.44 Comparison of depth averaged velocity field obtained from model morphology and that measured near the bed in laboratory Experiment Two, downstream of the obstruction and deposition mounds.....	76
Figure 3.45 System layout for prediction of lagoon equilibrium morphology.	77

Figure 3.46 Resultant morphology from application of self-organisation-based method. Flow was from right to bottom.	78
Figure 3.47 Global trend in energy dissipation as the lagoon system self-organises.	79
Figure 3.48 Global trend in energy dissipation as the channel obstruction system self-organises.	80
Figure 4.1 Model setup for (a) field sized lagoon and (b) the laboratory scale case study.	81
Figure 4.2 General methodology of optimisation routines.	83
Figure 4.3 Sketch of areas where global energy dissipation of the system are calculated.	84
Figure 4.4 Solving flow around a plate in a flume using optimisation methods.	87
Figure 4.5 Staggered central finite difference grid system.	87
Figure 4.6 Real genetic algorithm methodology.	89
Figure 4.7 Average crossover, where A, B, C and D are four different elevation values, and AC and BD are averages of these values.	90
Figure 4.8 One-point crossover, where A, B, C and D are four different elevation values.	90
Figure 4.9 Velocity pattern around a plate – comparison of (a) RGA and (b) BGA results in a short channel.	93
Figure 4.10 Comparison of BGA and RGA objective function values.	94
Figure 4.11 Simulated annealing methodology.	95
Figure 4.12 Comparison of RGA and SA optimisation models. The objective function value is given in log scale.	97
Figure 4.13 Velocity pattern around a plate – best results from (a) SA and (b) RGA in a long channel with a plate asymmetrically positioned.	98
Figure 4.14 Comparison of combined RGA-SA and SA optimisation models.	100
Figure 4.15 Initial random velocity pattern in a long channel around an asymmetrically positioned plate.	100
Figure 4.16 Velocity pattern around an asymmetrically positioned plate – obtained using (a) a standard hydrodynamic solver and (b) the RGA-SA optimisation model.	101
Figure 4.17 Example of field-sized unidirectional lagoon.	102
Figure 4.18 Path of objective function during GA optimisation.	103
Figure 4.19 Path of objective function found when an SA was employed after the GA shown in Figure 4.18 stagnated.	103
Figure 4.20 (a) Initial random morphology utilised in the GA optimisation model, (b) morphology after 200 GA generations, (c) morphology after the GA optimisation and (d) morphology after the SA optimisation.	104
Figure 4.21 Path of Objective Function Values for Sequential SAs.	106
Figure 4.22 Resultant morphology from the application of a GA and three SAs sequentially.	106
Figure 4.23 Morphology of lagoon with only global energy dissipation component in objective function value calculations.	107

Figure 4.24 Morphology of lagoon after SA using points closer to the boundaries for energy calculations.....	108
Figure 4.25 Morphology of lagoon using average water elevations and velocities after SA optimisation.....	108
Figure 4.26 Comparison of objective value paths using different energy dissipation descriptions.	109
Figure 4.27 Morphology of lagoon using critical velocity penalties only as objective function after GA.	109
Figure 4.28 Morphology of lagoon using critical velocity penalties only as objective function after SA.....	110
Figure 4.29 Path of objective function values for optimisation with and without global energy using GA only.	110
Figure 4.30 Enlargement of SA results for path of objective function values for optimisation with and without global energy.....	111
Figure 4.31 Description of flow reversal lagoon objective function definitions.	114
Figure 4.32 Best morphologies after 600 and 3000 generations for unidirectional and flow reversal GA optimisation.	115
Figure 4.33 Best morphologies for unidirectional and flow reversal SA optimisation at final temperature.	116
Figure 4.34 Aerial photograph of a small section of the River Murray mouth for comparison with the modelled morphology.	116
Figure 4.35 Path of Objective Function Value for GA optimisation.....	117
Figure 4.36 Path of Objective Function Value for SA optimisation.....	117
Figure 4.37 Best morphology for flow reversal second SA optimisation.....	118
Figure 4.38 Laboratory lagoon setup description.	119
Figure 4.39 Laboratory setup for a lagoon experiment.....	120
Figure 4.40 Equilibrium morphology after 75 hours of flow down the flume. IBL represents the initial average bed level.	121
Figure 4.41 Equilibrium morphology prediction using traditional process-based model. The initial average bed level was zero.	122
Figure 4.42 Path of objective function using a GA optimisation routine.	123
Figure 4.43 Further enhancement of the minimum objective function by the SA, after the stagnation of the GA routine.	124
Figure 4.44 Equilibrium morphology prediction after SA optimisation.....	124
Figure 5.1 Idealised salient equilibrium morphology associated with a shore-parallel detached breakwater.....	130
Figure 5.2 Comparison of equilibrium morphologies observed (a) before and (b) after shore-parallel breakwater insertion in a laboratory experiment. The shore-parallel detached breakwater was located at (0,0), with a width of 585mm.....	131
Figure 5.3 Typical randomly generated starting morphology.....	132
Figure 5.4 Description of random elevation prediction areas.....	133
Figure 5.5 General methodology of top-down model.....	136

Figure 5.6 Breakwater set-up after Nicholson et al. (1997).....	137
Figure 5.7 Examples of steady-state morphologies obtained using different 2DH process-based models – (a) DHI and (d) STC (after Nicholson et al., 1997).	137
Figure 5.8 Morphology associated with a detached breakwater after 14 days using a 2DH model (after Zyserman and Johnson, 2002).....	138
Figure 5.9 The resultant morphology after GA optimisation using a top-down model and variable elevation limits of 60% of the water depth in deposition and 30% in erosion.	139
Figure 5.10 Initial and final wave pattern for (a) a sloping beach and (b) the morphology shown in Figure 5.9.	140
Figure 5.11 Initial and final velocity pattern for (a) a sloping beach and (b) the morphology shown in Figure 5.9.....	140
Figure 5.12 Comparison of equilibrium morphologies obtained using a top-down model with variable limits and different seeds to generate the initial populations of random morphologies which the model then optimises.....	141
Figure 5.13 Comparison of objective function paths for different random starting morphologies, where (a), (b), (c) and (d) correspond to the morphologies shown in Figure 5.12.	142
Figure 5.14 Definition of variable limits.	143
Figure 5.15 Definition of fixed limits.....	144
Figure 5.16 Resultant morphology using variable limits of (a) 45% water depth for erosion and 20% water depth for deposition, and (b) 60% and 30% respectively.....	144
Figure 5.17 Path of objective function using variable limits of (a) 45% water depth for erosion and 20% water depth for deposition, and (b) 60% and 30% respectively. ...	145
Figure 5.18 Resultant morphology using a top-down model with fixed random elevation limits of ± 2.5 m.....	146
Figure 5.19 Path of the objective function using a top-down model with fixed random elevation limits of ± 2.5 m.	147
Figure 5.20 Comparison of results using variable random elevation limits and a fixed morphological grid size of (a) 80m and (b) 20m.	148
Figure 5.21 Path of objective function using fixed morphological grid size of (a) 80m and (b) 20m.....	148
Figure 5.22 Equilibrium morphology predicted using a GA, with a (a) 50m morphological grid size and (b) using a combined GA-SA with a 30m morphological grid size. Fixed limits of 4m deposition and 3m erosion, either side of the initial slope were imposed.....	150
Figure 5.23 (a) Path of the objective function using (a) a GA and (b) a follow on SA, with a 30m morphological grid size and random elevation limits of 4m deposition and 3m erosion.....	151
Figure 5.24 Equilibrium morphology predicted using a GA, with a 50m morphological grid size and random elevation limits of (a) ± 1.5 m and (b) 4m deposition and 3m erosion.....	152
Figure 5.25 Path of objective function using (a) a +4m and -3 m change in elevation limit and (b) ± 1.5 m elevation limit.	152

Figure 5.26 Resultant morphology using top-down model with minimum variance inclusion in objective function.....	154
Figure 5.27 Path of objective function using top-down model with minimum variance inclusion in objective function.....	154
Figure 5.28 Resultant morphology with the inclusion of minimum variance and a fixed morphological grid size of 40m.	155
Figure 5.29 Path of objective function using fixed morphological grid size of 40m, with minimum variance inclusion in objective function.....	155
Figure 5.30 Comparison of results using a fine morphological grid (a) with and (b) without minimum variance included in the objective function calculations.....	156
Figure 5.31 Path of objective function using fixed morphological grid size of 20m, (a) with minimum variance and (b) without minimum variance inclusion in the objective function.	157
Figure D.1 Flume set-up for lagoon laboratory experiments.....	305
Figure D.2 Contour plot of the equilibrium bed morphology of Experiment One. The flow direction was from left to right.	306
Figure D.3 Contour plot of the equilibrium bed morphology of Experiment Two. The flow direction was from left to right.	307
Figure D.4 Morphology of Experiment Two after six hours, flow was from the top right corner to the bottom left corner. Notice the ripple patterns marking the flow path..	307
Figure D.5 Morphology of Experiment Two after six hours, flow was from the bottom right corner to the top left corner. Notice the ripple patterns marking the flow path.	308
Figure D.6 Contour plot of the equilibrium bed morphology of Experiment Three. The flow direction was from left to right.	309
Figure D.7 Contour plot of the equilibrium bed morphology of Experiment Four. The flow direction was from left to right.	310
Figure E.1 General breakwater laboratory set-up. (The wave direction was at a slight angle of approximately four degrees.)	312
Figure E.2 Calibration chart for initial beach formation in Experiment Three.....	312
Figure E.3 Example of data used in zero up crossing method to obtain average wave height and period information from Experiment Three.....	313
Figure E.4 Contour plot of the initial equilibrium bed morphology of Experiment One. The 120mm contour line represents the SWL. The wave direction was from bottom to top.	316
Figure E.5 Contour plot of the bed morphology of Experiment One after (a) ten minutes, (b) thirty minutes and (c) one hour of wave action on a breakwater protected beach. The wave direction was from bottom to top.	316
Figure E.6 Contour plot of the bed morphology of Experiment One after (a) five hours, (b) six hours,(c) ten hours and (d) twelve hours of wave action on a breakwater protected beach. The wave direction was from bottom to top.....	317
Figure E.7 Contour plot of the bed morphology of Experiment One after (a) 16 hours and (b) 17 hours of wave action on a breakwater protected beach. The wave direction was from bottom to top.	317

Figure E.8 Difference in bed morphology of Experiment One between (a) 16 hour and 17 hour and (b) initial and 17 hour morphologies. The wave direction was from bottom to top..... 318

Figure E.9 Contour plot of the final equilibrium bed morphology of Experiment One after 17 hours of wave action towards the breakwater. The 120mm contour line represents the SWL. The wave direction was from bottom to top..... 319

Figure E.10 Difference in complete bed morphology of Experiment One between initial and 17 hour morphologies. The wave direction was from bottom to top..... 319

Figure E.11 Contour plot of the (a) initial and (b) final equilibrium bed morphology of Experiment Two. The dashed line, just above the 60mm contour line represents the SWL. The wave direction was from bottom to top. The placement of the breakwater after this equilibrium beach was formed is alluded to by the black and the white rectangle in the bottom of the plot for (a) and (b) respectively. 320

Figure E.12 Difference in complete bed morphology of Experiment Two between initial and 5.5 hour morphologies. The wave direction was from bottom to top. The dashed lines represent the initial and final SWL. 321

Figure E.13 Contour plot of the (a) initial and (b) final equilibrium bed morphology of Experiment Three. The wave direction was from bottom to top. 322

Figure E.14 Contour plot of the bed morphology of Experiment Three after (a) ten minutes, (b) thirty minutes, (c) one hour and (d) 3 hours of wave action (including 1 hour of storm wave action) on a breakwater protected beach. The wave direction was from bottom to top. 323

Figure E.15 Contour plot of the bed morphology of Experiment Three after (a) 8.5 hours, (b) 13 hours, (c) 18 hours, (d) 23 hours, (e) 24.5 hours and (f) 25 hours of wave action on a breakwater protected beach. The wave direction was from bottom to top. 324

List of Tables

Table 3.1	Flow conditions used for each of the six experiments.....	45
Table 3.2	Sampling scheme for velocity measurements.....	46
Table 3.3	Measured water depths and flow rates with corresponding calculated velocities.	52
Table 3.4	Velocities measured when Experiment Six had reached an equilibrium configuration, along transverse transects, as shown in Figure 3.12, using the ADV meter.	53
Table 3.5	Flow characteristics of laboratory experiments.	59
Table 3.6	Measured water depths and flow rates with corresponding calculated velocities.	62
Table 3.7	Velocities measured after formation of equilibrium morphology along transverse transects, as in Figure 3.25, using the ADV meter.	62
Table 3.8	Brier Skill Score interpretation guidelines (after Sutherland et al., 2004).....	64
Table 4.1	Values obtained using GA or SA only and RGA-SA optimisation combination.	99
Table 4.2	Comparison table for values of RMAE velocity, as suggested by van Rijn et al. (2003).....	101
Table B.1	Sample of some GA sensitivity analyses.	203
Table B.2	Sample of some SA and combination sensitivity analyses.	204
Table D.1	Flow conditions used for each of the five experiments.	305
Table E.1	Initial and final distances to the SWL for each of the experiments.....	314
Table E.2	Initial distances from the breakwater to the SWL for each of the experiments.	314

List of Symbols

Symbol	Quantity	SI Unit
a	Elasticity of acceptance	-
$A_{ij}(c)$	Metropolis criterion	-
B	Initial or baseline laboratory measurement	m
c	Current temperature	-
c_0	Initial fitness	m^3s^{-1}
d	Mean depth of water	m
D	Local water depth	m
D_{35}	35th percentile of sediment size	m
d_{50}	50th percentile of sediment size	m
d_{90}	90th percentile of sediment size	m
E_e	Average specific energy	m
E_s	Global specific energy contribution	m
f	Objective function value	m^3s^{-1}
f_w	Wave friction factor	-
g	Acceleration due to gravity	ms^{-2}
H	Height of the landscape	m
h	Water depth	m
k_s	Roughness length of the bed	
LL	Lower limit of each decision variable	m
M	Mass	kg
MSE	Mean squared error	m
η	Surface elevation with respect to a datum	m
OF	Overall objective function value	m^3s^{-1}
P_a	Excess steepness penalty	m
P_d	Excess velocity sediment depositing penalty	ms^{-1}
P_e	Excess velocity sediment eroding penalty	ms^{-1}
ρ_s	Density of sediment	kgm^3
ρ	Fluid density	kgm^3
Q	Thermal or heat energy	J
q_{ij}	Sediment transport	m^3s^{-1}
q_t	Volumetric total sediment transport rate	m^2s^{-1}
Q_s	Volumetric transport rate of sediment	m^3s^{-1}
RND	Random number between 0 and 1.	-

Symbol	Quantity	SI Unit
S	Entropy	JK ⁻¹
$SS1$	Random or a predetermined step size	-
T	Period of oscillation	s
T	Temperature	K
t_1	Starting temperature	-
t	Time	s
$\hat{\tau}_0$	Amplitude of τ_0	Pa
τ_0	Shear stress on the bed	Pa
u	Velocity vector component in the x direction	ms ⁻¹
UL	Upper limit of each decision variable	m
U_∞	Amplitude of u_0	ms ⁻¹
\bar{U}	Mean value of u over a vertical	ms ⁻¹
\bar{u}_*	Time-mean value of u_*	ms ⁻¹
\bar{u}_{cr}	Critical mean flow velocity	ms ⁻¹
ν	Kinematic viscosity	m ² s ⁻¹
v	Velocity vector component in the y direction	ms ⁻¹
var	Variance in elevation	m
X	Laboratory measurement	m
x	Streamwise distance	m
Y	Numerical model prediction	m
z_0	Baseline elevation	m
z_{crit}	Angle of repose elevation	m
z_{ij}	Predicted elevation	m
λ_p	Bed porosity	-
θ	Scaling factor	-
ψ	Scalar factor	-

List of Abbreviations

ADV	Acoustic Doppler Velocity Meter
BGA	Binary genetic algorithm
BSS	Brier Skill Score
2DH	Two- dimensional, depth averaged horizontal
DHI	Danish Hydraulic Institute
GA	Genetic algorithm
Q3D	Quasi-three-dimensional
RGA	Real genetic algorithm
RMAE	Relative mean absolute error
SA	Simulated annealing
STC	Service Technique Central des Ports Maritimes et des Voies Navigables