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**AXIAL COMPRESSIVE BEHAVIOR OF FRP-CONFINED CONCRETE:
EXPERIMENTAL TEST DATABASE AND A NEW DESIGN-ORIENTED MODEL**

Togay Ozbakkaloglu¹ and Jian C. Lim²

ABSTRACT

A large number of experimental studies have been conducted over the last two decades to understand the behavior of FRP-confined concrete columns. This paper presents a comprehensive test database constructed from the results of axial compression tests on 832 circular FRP-confined concrete specimens published in the literature. The database was assembled through an extensive review of the literature that covered 3042 test results from 253 experimental studies published between 1991 and the middle of 2013. The suitability of the results for the database was determined using carefully chosen selection criteria to ensure a reliable database. This database brings reliable test results of FRP-confined concrete together to form a unified framework for future reference. Close examination of the test results reported in the database led to a number of important observations on the influence of important parameters on the behavior of FRP-confined concrete. A new design-oriented model that was developed to quantify these observations is presented in the final part of the paper. It is shown that the predictions of the proposed model are in close agreement with the test results and the model provides improved predictions of the ultimate conditions of FRP-confined concrete compared to any of the existing models.

KEYWORDS: FRP-confined concrete; A. Fibers; B. Plastic deformation; B. Strength; D. Mechanical testing

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1. INTRODUCTION

Axial compressive behavior of FRP-confined concrete has received significant attention over the last two decades, and it is now well understood that the confinement of concrete with fiber reinforced polymer (FRP) composites can substantially enhance concrete strength and deformability. A large number of experimental studies have produced over 3000 test results on FRP-confined concrete and resulted in the development of over 90 axial stress-axial strain models, 88 of which were recently reviewed and assessed in Ozbakkaloglu et al. [1]. It became evident from the results of the assessment reported in Ozbakkaloglu et al. [1] the performances of a large proportion of the existing models were compromised when assessed against a large test database with a parametric range that is much wider than the databases used in the development of these models. These observations clearly revealed the need for an extensive and reliable experimental test database of FRP-confined concrete for the development of models of higher accuracy.

In this paper, a carefully prepared database of circular FRP-confined concrete specimens tested under monotonic uniaxial compression is presented. The database was assembled through an extensive review of the literature that catalogued 3042 test results from 253 experimental studies published between 1991 and the middle of 2013. These results were then assessed according to criteria that had been critically determined to establish a reliable database. Assessment using these criteria resulted in a final database of 832 test results from 99 different sources. This database serves as a valuable reference document for: i) future model development and verification; ii) assessment of existing models; and iii) future database establishment. The important factors that influence the overall behavior of FRP-confined concrete, as identified from the results reported in the comprehensive database, are then discussed. In the final part of the paper, a new design-oriented model developed using the database to predict the ultimate condition of FRP-confined concrete is presented.

2. CONSTRUCTION OF EXPERIMENTAL TEST DATABASE

2.1. Previous databases

Due to the inherent complexity of the behavior of FRP-confined concrete, test databases serve as a vital verification tool in assessing the performance of a model. Recognition of the importance of systematically collecting and categorizing the existing test results has led to a number of previous attempts to develop test databases for FRP-confined concrete. All relevant details of these previous databases are summarized in Table 1. The earlier databases reported by Lam and Teng [2, 3], De Lorenzis and Tepfers [4] and Bisby et al. [5] are extensive and include the majority of the experimental data with sufficient detail that were available at the time the databases were published. More recently, Turgay et al. [6] compiled a database of carbon FRP-confined concrete specimens and Realfonzo and Napoli [7] reported a fairly large database of carbon and glass FRP-wrapped specimens. However, a comprehensive review of the literature indicated that a large number of the currently available test results summarized in Table 2 were not included in any of the existing databases.

2.2. Selection criteria for the new database

The suitability of the results for the database was assessed using carefully established selection criteria to ensure both the reliability and consistency of the test data. This resulted in a final database of 832 datasets, which makes it by far the most comprehensive database reported in the literature. The test results included in this database, summarized in Table 2 and presented in Tables 3 to 7, met the following requirements:

- 1) Only the specimens with unidirectional fibers orientated in the hoop direction were included in the database.
- 2) Specimens with transverse and/or longitudinal steel or internal FRP reinforcement were excluded.

- 3) Only the specimens that were confined with continuous FRP jackets were included. Specimens with partial wrapping (i.e., FRP strips) were excluded.
- 4) Specimens with a height-to-diameter (H / D) ratio greater than three were excluded from the database to eliminate the influence of specimen slenderness.
- 5) Specimens with unconfined concrete compressive strengths greater than 55 MPa were excluded to limit the database to only normal-strength concrete.
- 6) Only the specimens that failed due to FRP rupture at the ultimate condition were included. Specimens that failed prematurely due to other types of failure, such as FRP shell debonding or premature failure due to excessive eccentricity were excluded.
- 7) Specimens for which the ultimate conditions were not recorded accurately due to inadequate testing equipment or instrumentation errors were excluded.
- 8) Specimens reported with insufficient details in regards to material and geometric properties were excluded.

The specimens that satisfied the above conditions, and hence were included in the test database, were then subjected to an additional set of conditions to establish their suitability for their inclusion in the assessment of the existing models and development of the new model. The specimens with compressive strengths (f'_{cc}) and ultimate axial strains (ϵ_{cu}) that deviated significantly from the global trends of relevant strength and strain enhancement ratios (i.e. more than $\pm 40\%$ of f'_{cc} / f'_{co} and $\pm 70\%$ of $\epsilon_{cu} / \epsilon_{co}$) were excluded in the model assessment and development. The specimens that were excluded from the calculations of the strength and strain enhancement ratios (f'_{cc} / f'_{co} and $\epsilon_{cu} / \epsilon_{co}$) are marked respectively with the superscripts 's' and 'a' in Tables 3 to 7. Furthermore, the specimens with hoop rupture strain reduction factors (k_ϵ) that deviated significantly from the average values of the corresponding material (i.e. more than $\pm 20\%$ of average k_ϵ) are marked with the superscript '^' in Tables 3 to 7, and they were excluded in the development of the expression for the hoop rupture strain reduction factor (k_ϵ). In addition to these, datasets from specimens exhibiting a stress-strain

curve with a descending second branch (marked with superscript ‘*d*’ in database tables) and ones from specimens having tubes that were fabricated using an automated manufacturing method (marked with superscript ‘*fm*’ in database tables) were also excluded in the model development and assessment to limit the investigation to specimens with ascending second branches and manually manufactured FRP jackets.

3. NEW TEST DATABASE

The complete test database assembled in the present study is displayed in Tables 3 to 7. The database consists of the following information for each specimen: confinement technique (wrapped or tube-encased concrete); specimen geometric properties (diameter D and height H); unconfined concrete strength (f'_{co}) and strain (ϵ_{co}); material properties of the FRP shell (elastic modulus E_{frp} , tensile strength f_{frp} , total thickness t_{frp}); material properties of the fibers used in the FRP shell (elastic modulus E_f , tensile strength f_f , total thickness t_f); compressive strength (f'_{cc}) and ultimate axial strain (ϵ_{cu}) of confined concrete, and average FRP hoop strain at rupture ($\epsilon_{h,rupt}$); and hoop rupture strain reduction factor based on fiber properties (k_{ϵ_f}) and FRP material properties ($k_{\epsilon_{frp}}$).

The test data presented in the database were sorted into eight groups based on two main confinement parameters: confinement technique (wraps or tubes) and type of FRP material [carbon FRP (CFRP); S- or E-glass FRP (GFRP); aramid FRP (AFRP); high-modulus carbon FRP (HM CFRP); or ultra-high-modulus carbon FRP (UHM CFRP)]. 755 specimens in the database were FRP-wrapped, whereas 77 specimens were confined by FRP tubes. 495 of the specimens were confined by CFRP; 206 by GFRP; 79 by AFRP; 40 by HM CFRP; and 12 by UHM CFRP.

The results of FRP-wrapped specimens are presented in Tables 3 to 6, categorized according to fiber type, and the results of all FRP tube-encased specimens are given in Table 7. It is worthwhile noting that for some of the datasets, a single entry in Tables 3 to 7 may represent the average results

of more than one nominally identical specimen, as reported in the original study. These datasets are clearly marked in Table 2. In addition, a group of unbonded-wrapped specimens tested by Harries and Carey [8], Mirmiran et al. [9], Mastrapa [10] and Matthys et al. [11] were grouped under the category of tube-encased specimens in the database. Furthermore, except for the datasets from Saafi et al. [12], Hong and Kim [13] and Ozbakkaloglu and Vincent [14], all the datasets included in the database tables were obtained from specimens that were confined by FRP shells (wraps or tubes) manufactured using a manual hand lay-up technique. The specimens of Ozbakkaloglu and Vincent [14] and Hong and Kim [13], on the other hand, were confined by FRP tubes that were manufactured using an automated filament winding technique; and the specimens of Saafi et al. [12] were confined with FRP tubes supplied by a manufacturer, with no specific manufacturing method reported in the source document. These datasets are marked with a superscript '*fm*' in Table 7 to highlight the fact that the FRP shells of these specimens were manufactured using an automated manufacturing method rather than a manual one.

The diameters of the specimens (D) included in the test database varied between 47 and 600 mm, with the majority of the specimens having a diameter of 150 mm. The unconfined concrete strength (f'_{co}) and strain (ϵ_{co}), as obtained from concrete cylinder tests, varied from 6.2 to 55.2 MPa and 0.14% to 0.70%, respectively. The actual confinement ratio, defined as the ratio of the actual ultimate confining pressure to the unconfined concrete strength ($f_{lu,a} / f'_{co}$), varied from 0.02 to 4.74. The FRP material properties reported in the database were obtained either from the material test results (i.e., coupon or ring splitting tests) reported in the original study or the specifications provided by the manufacturers. The specimens with FRP properties that differed significantly from the reference properties of the corresponding material were marked with the superscript '*m*' in Tables 3 to 7, to point to potential errors in these properties.

3.1. Material properties of fibers and FRP composites reported in the database

In FRP-confined circular concrete sections, the lateral confining pressure (f_l) provided by the FRP shell can be assumed to be uniformly distributed around the circumference (Figure 1). The confinement exerted by the FRP shell on the concrete core is passive; that is, this pressure arises as a result of the lateral expansion of the concrete under axial compression. As the FRP shell is subjected to tension along its hoop direction, the confining pressure (f_l) increases proportionally with the lateral expansion until the eventual failure of the system when the FRP shell ruptures. Based on the deformation compatibility between the confining shell and the concrete surface and assumption of a uniform confining pressure distribution, the lateral confining pressure applied to the concrete by the FRP shell at ultimate (f_{lu}) can be theoretically calculated from Eq. 1 as a function of the ultimate tensile strain of the fibers (ε_f).

$$f_{lu} = \frac{2E_f t_f \varepsilon_f}{D} \quad \text{Eq. 1}$$

However, it has been well documented that the ultimate strain measured on the FRP shell at the time of FRP hoop rupture ($\varepsilon_{h,rupt}$) is often lower than the ultimate tensile strain of the fibers (ε_f) or FRP material (ε_{frp}) (e.g. [3, 4, 8, 9, 11, 15-25]). Several causes have been given for the observed differences between hoop rupture strains and material ultimate tensile strains, including: (i) the quality of workmanship; (ii) overlaps of fiber sheets in the FRP shell; (iii) manufacturing imperfections (e.g., misalignment of fibers); (iv) shrinkage of the concrete (for FRP tube-encased concrete); (v) localized or non-uniform effects caused by imperfections in FRP shells and/or heterogeneity of cracked concrete; (vi) load eccentricities caused by specimen imperfections and/or test setup imprecisions; (vii) multiaxial stress condition generated on the FRP shell; and (viii) effect of the curvature of the FRP shell.

To establish the relationship of the hoop rupture strain of the FRP shell ($\varepsilon_{h,rupt}$) and the ultimate tensile strain of the material (ε_f or ε_{frp}), a strain reduction factor (k_ε) was defined by Pessiki et al.

[17] (Eq. 2). Lam and Teng [3] then defined a term called the actual confining pressure ($f_{lu,a}$) (Eq. 3), by replacing the ultimate tensile strain (ε_f or ε_{frp}) of the material with the hoop rupture strain of the FRP shell ($\varepsilon_{h,rupt}$) in Eq. 1.

$$\varepsilon_{h,rupt} = k_{\varepsilon,f} \varepsilon_f \text{ or } \varepsilon_{h,rupt} = k_{\varepsilon,frp} \varepsilon_{frp} \quad Eq. 2$$

$$f_{lu,a} = \frac{2E_f t_f \varepsilon_{h,rupt}}{D} \quad Eq. 3$$

In Eq. 2, due attention should be given to ensure that the strain reduction factors ($k_{\varepsilon,f}$ or $k_{\varepsilon,frp}$) are used consistently with the corresponding ultimate material tensile strain (ε_f or ε_{frp}). In the studies examined, the properties of the FRP confinement systems were reported in several different ways. The reported details included: (i) the manufacturer specified properties of fibers; (ii) the manufacturer specified properties of FRP (iii) FRP properties as determined from flat coupon tests based on measured coupon thickness; (iv) FRP properties as determined from flat coupon tests based on nominal fiber sheet thickness; and (v) FRP properties as determined from ring-splitting tests. Only a small number of studies [10, 26, 27] reported the FRP properties obtained from ring-splitting tests, and the majority of the studies provided the properties obtained from flat coupon tests or supplied by manufacturers. As for the FRP properties obtained from flat coupon tests, in some of the studies [18, 28-36] the elastic moduli (E_{frp}) and tensile stresses (f_{frp}) were calculated based on nominal fiber thickness instead of the measured thickness of flat FRP coupons. The datasets from these studies are marked with the superscript 't' in Tables 3 to 7.

In the database provided in Tables 3 to 7, due attention was given to establish a clear distinction between the fiber and FRP properties in the reported values of the elastic modulus (E_f or E_{frp}), tensile strength (f_f or f_{frp}), and total thickness (t_f or t_{frp}) of the confining material. In the model assessment and development, if a dataset included both fiber and FRP properties, the model

predictions were based on the fiber properties, unless the fiber properties were marked with the superscript ' f ' indicating they were either incomplete or established to be inaccurate based on the analysis of the database.

3.2. FRP confinement technique

A potentially important distinction, often recognized by the models assessed in the present study, is the one that is made between FRP-wrapped and FRP tube-encased specimens. Previously, both Mirmiran et al. [9] and Lam and Teng [2] reported that there was no significant difference between the behaviors of FRP-wrapped and FRP tube-encased concrete specimens. On the other hand, Saafi et al. [12] concluded that the ultimate condition of FRP-confined concrete was influenced by the adopted confinement technique.

In the present study, the test database was sorted into two categories and the results of the FRP-wrapped and FRP tube-encased specimens are presented in separate tables. Tables 3 to 6 show the results for FRP-wrapped concrete, whereas Table 7 reports the results for FRP tube-encased specimens. Comparison of the trends of the strength and strain enhancement ratios of FRP-wrapped specimens with those of FRP tube-encased specimens (Figures 2 and 3) indicate that there are noticeable differences between the ultimate conditions of these two groups of specimens. However, it is not possible to draw a definitive conclusion based on these observations, as in the database the FRP-wrapped specimens significantly outnumber the FRP tube-encased specimens. It is possible that observed differences might have been caused partly or entirely by the differences in the data ranges and specimen distributions between the two sets of test results.

3.3. Type of FRP material

Several previous studies have focused on the influence of the types of FRP materials on the behavior of FRP-confined concrete (e.g., [3, 18, 37, 38]). Most of these studies reported that, for a

given confinement ratio ($f_{lu,a} / f'_{co}$), the compressive strength (f'_{cc}) of FRP-confined concrete is influenced only marginally by the type of FRP material; whereas, it was found that the ultimate strain of FRP-confined concrete (ϵ_{cu}) is highly sensitive to the material properties of the confining FRP. It is now understood that, for a given confinement ratio ($f_{lu,a} / f'_{co}$), the ultimate axial strain of the FRP-confined concrete increases with the increased ultimate tensile strain (ϵ_f or ϵ_{frp}) of the materials used in confining it. This understanding is supported by the trends of the test results reported in the database of the present study [Figures 4 (a) and (b)]. It is evident from these figures that the trend lines of the strain enhancement ratios are sensitive to the type of FRP, whereas the strength enhancement ratio is not highly influenced by changes in the type of FRP.

Given its direct influence on the actual confinement ratio ($f_{lu,a} / f'_{co}$) and therefore the ultimate condition of FRP-confined concrete, it is obvious that the accurate determination of hoop rupture strains plays an instrumental role in the prediction of the ultimate condition of FRP-confined concrete. The average values of the strain reduction factors determined from the database reported in the present study (Table 8), point to the influence of the fiber type on the strain reduction factor ($k_{c,f}$) and hence on the hoop rupture strains. This influence, which was also reported previously in Ozbakkaloglu and Akin [39] and Dai et al. [40], is discussed further later in the paper.

3.4. Instrumentation details of specimens reported in the database

The ultimate axial strains (ϵ_{cu}) and FRP hoop rupture strains ($\epsilon_{h,rupt}$) in the database are the average values obtained by strain gauges or deformation measuring devices. In the previous studies, a number of measurement methods were used to record the ultimate axial strains, including: (i) strain gauges attached to the surface of FRP shells (AS); (ii) deformation measuring devices, such as linear variable deformation transducers (LVDTs), extensometers or dial gauges mounted between each platen of the axial compression test machine (AFL); and (iii) measuring devices mounted within a certain gauge length along the height of the specimens (AML).

Similarly, different measuring methods have been used in measuring the hoop strains, including methods (i) and (iii) noted above, with strain gauges or measuring devices oriented in the hoop direction. Information regarding the specific methods in the measurement of both of these strains is reported in the final column of Table 2 for each study included in the database. For the specimens where multiple hoop strain gauges were used, such as the specimens tested by Lam and Teng [18], Smith et al. [36], and Wu and Jiang [24], the average values of the strain gauge measurements have been recorded in the database. In the calculations of the average values, due attention was given to the exclusion of inconsistent strain gauge readings, such as those coming from the overlap regions of FRP sheets.

3.5. Test database size and scatter

Test databases inherently produce a scatter of test results. Bisby et al. [5] reported that the scatter of test results caused an average absolute error (*AAE*) of no less than 13% for the strength enhancement ratio (f'_{cc} / f'_{co}) and 35% *AAE* for the strain enhancement ratio ($\epsilon_{cu} / \epsilon_{co}$) in their database of approximately 200 datasets. The natural scatter of the database reported in the present study was lower than these thresholds, with *AAE* values of 11% and 23% for strength and strain enhancement ratios respectively, even though the size of the database was significantly larger with 832 datasets. The relatively low scatter of this database was achieved through the use of carefully chosen selection criteria in the collection of the test data, as outlined previously, to ensure consistency and reliability.

As was reported previously in De Lorenzis and Tepfers [4], variability in material properties of the test specimens, such as the stiffness of the FRP confining shell, the type and size of aggregates used in the concrete mix, and the mix proportions and moisture content of the concrete, contribute to the scatter found in test databases. As discussed in Section 3.4, the differences in the instrumentation of

the specimens and the setups used in testing them also contribute significantly to scatter. In particular, the two key ultimate condition properties, the ultimate axial strain (ϵ_{cu}) and hoop rupture strain ($\epsilon_{h,rupt}$), are highly sensitive to the instrumentation arrangement used in specimen testing. Figure 5 shows the variation of the strain enhancement ratios ($\epsilon_{cu} / \epsilon_{co}$) with the actual confinement ratios ($f_{lu,a} / f'_{co}$), as obtained using different axial strain measurement methods. Only CFRP-wrapped specimens, which formed the largest sub-group in the database, were included in Figure 5 in order to eliminate the additional influences caused by differences in the type of FRP and the method of confinement. Differences in the trendlines shown in Figure 5 suggest that the recorded ultimate axial strains may be influenced by the measurement method used in their determination.

Similarly, it should be expected that the average recorded hoop rupture strains ($\epsilon_{h,rupt}$) will be influenced by the number and placement of strain gauges used in the measurement of these strains. As reported originally in Lam and Teng [3], hoop strains measured within the overlap regions of the FRP jackets are known to be lower than those measured elsewhere around the perimeter of the same FRP jacket. It follows, therefore, that the differences in the hoop strain gauge arrangements of the specimens included in the database are one of the main reasons for the inherent scatter in the hoop rupture strain data reported in the database.

4. A NEW DESIGN-ORIENTED MODEL

This section presents a new design-oriented model to predict the ultimate condition of FRP-confined concrete. The model contains closed-form expressions that were developed using the test database presented in Tables 3 to 7. Not all the datasets included in the database contained all the relevant details required for the development of all the components of the model. Furthermore, as discussed previously, the results that failed to satisfy the criteria outlined in Section 2.2 were excluded from model development. The total number of datasets that were used in the calibration of the hoop strain

reduction factor (k_ε), strength enhancement coefficient (k_I), and strain enhancement coefficient (k_2) are given in Tables 8 to 10, respectively.

4.1. Hoop rupture strain of FRP-confined concrete

Table 8 provides the values of the strain reduction factors ($k_{\varepsilon,f}$ and $k_{\varepsilon,frp}$) determined from the database presented in this paper. Using these values together with the ones obtained from the tests of over 250 FRP-confined high-strength concrete specimens reported in Lim and Ozbakkaloglu [41], the key parameters that influence the strain reduction factor were indentified. It was found that the increase in either the compressive strength of concrete (f'_{co}) or elastic modulus of confining fibers (E_f) result in a decrease in the recorded hoop rupture strains ($\varepsilon_{h,rupt}$) and hence in the strain reduction factors ($k_{\varepsilon,f}$ and $k_{\varepsilon,frp}$). The former influence was first reported in Ozbakkaloglu and Akin [39] and it can be attributed to the increased concrete brittleness with increasing concrete strength, which alters the concrete crack patterns from heterogenic microcracks to localized macrocracks. The observed dependence of the strain reduction factor to the type of confining fibers was previously noted in Ozbakkaloglu and Akin [39] and Dai et al. [40]. Further observations from the comprehensive database reported in this study on the relationship between the elastic modulus of confining fibers (E_f) and recorded hoop rupture strains ($\varepsilon_{h,rupt}$) indicate that the influence of the fiber brittleness on the strain reduction factor resembles the aforementioned influence of the concrete brittleness on the same factor. The statistical quantification of the influences of these two parameters resulted in the expression given in Eq. 4 for the calculation of the hoop rupture strain reduction factor of fibers ($k_{\varepsilon,f}$). The expression is capable of predicting the $k_{\varepsilon,f}$ for FRP-confined concrete with an unconfined concrete strength up to 120 MPa, and confined by any of GFRP, AFRP, CFRP, HM FRP or UHM CFRP.

$$k_{\varepsilon,f} = 0.9 - 2.3f'_{co} \times 10^{-3} - 0.75E_f \times 10^{-6} \quad \text{Eq. 4}$$

where f'_{co} and E_f are in MPa.

4.2. Compressive strength of FRP-confined concrete

The proposed compressive strength expression (Eq. 5) incorporates several important parameters which were previously identified in Ozbakkaloglu et al. [1]. The strength enhancement effect at the first peak stress (f'_{cl}) of the stress-strain response is captured using Eq. 6 as a function of the confinement stiffness of the FRP shell (K_l). In order for the FRP-confined concrete to achieve a strain-hardening response after the first peak stress (f'_{cl}), the stiffness of the FRP reinforcing shell (K_l) has to exceed a minimum threshold value. The confining pressure at the corresponding condition is defined as the threshold confining pressure (Eq. 7) and can be estimated based on the corresponding hoop strain (ε_{l1}) (Eq. 8) in the FRP shell. As the proposed strength expression (Eq. 5) is only applicable to specimens that achieves strength enhancement after the first peak stress (f'_{cl}), the expression satisfies the confinement stiffness threshold requirement as given in Eq. 9. The prediction of the strength enhancement effect after the first peak stress (f'_{cl}) is based on the net confining pressure, that is, the reduced actual confining pressure ($f_{lu,a}$) after subtraction of the threshold confining pressure (f_{lo}). The strength enhancement effect generated by the net confining pressure is quantified using the coefficient of strength enhancement (k_1) in Eq. 5. It was found that establishing the compressive strength expression based on the net confining pressure yields an improved model prediction especially for specimens with higher unconfined concrete strengths (f'_{co}).

$$f'_{cc} = c_1 f'_{co} + k_1 (f_{lu,a} - f_{lo}) \quad \text{Eq. 5}$$

$$c_1 = \frac{f'_{cl}}{f'_{co}} = 1 + 0.0058 \frac{K_l}{f'_{co}} \quad \text{Eq. 6}$$

$$f_{lo} = K_l \varepsilon_{l1} \quad \text{Eq. 7}$$

$$\varepsilon_{l1} = \left(0.43 + 0.009 \frac{K_l}{f'_{co}} \right) \varepsilon_{co} \quad \text{Eq. 8}$$

$$K_l = \frac{2E_f t_f}{D} \text{ and } K_l \geq f'_{co}{}^{1.65} \quad \text{Eq. 9}$$

where K_l and f'_{co} are in unit MPa. It should be noted that the expression given in Eq. 5 is intended for FRP-confined concrete exhibiting a stress-strain curve with an ascending second branch. To this end, a statistically established condition equation, which is based on the observed influence of the confinement stiffness (K_l) and concrete strength (f'_{co}) on the trend of the second branch, is given in Eq. 9 as part of the proposed expression.

Table 9 summarizes the values of the strength enhancement coefficient (k_l) calibrated from the database for different types of FRP materials and confinement methods. It should be noted that the k_l values of the UHM CFRP-wrapped and HM and UHM CFRP tube-encased specimens are not presented in the table due to unreliability of the results caused by very limited number of available datasets. Additional experimental results are required to be able to determine reliable k_l values for these specific subgroups. In the absence of these results, the average value of k_l established from the database (i.e. $k_l = 3.2$) can be used for conservative estimates of these specimens.

4.3. Ultimate axial strain of FRP-confined concrete

As reported in Ozbakkaloglu et al. [1] almost all of the better performing ultimate strain enhancement expressions proposed in the literature have non-linear forms in their predictions of the strain enhancement ratio ($\varepsilon_{cu} / \varepsilon_{co}$) as a function of confinement ratios ($f_{lu,a} / f'_{co}$) (e.g. [42, 43]). This is due to the dependency of the strain enhancement ratio ($\varepsilon_{cu} / \varepsilon_{co}$) to the ultimate tensile strain of the material (ε_f or ε_{frp}), in addition to the confinement ratio ($f_{lu,a} / f'_{co}$), as was pointed out in a number of previous studies [3, 18, 37, 39]. To develop unified strain enhancement expressions for different

types of FRP materials in model presented in this paper, the axial strain (ε_{cu}) was quantified as a non-linear function of the confinement stiffness (K_l), hoop rupture strain ($\varepsilon_{h,rupt}$), and unconfined concrete strength (f'_{co}), as given in Eq. 10. In the equation, k_2 is the coefficient of strain enhancement and c_2 (Eq. 11) is the concrete strength factor, which is incorporated into the proposed expression to allow for the change in the shape of the stress-strain curve of unconfined concrete with the variation of concrete strength (f'_{co}). In Eq. 10, the axial strain corresponding to the unconfined concrete peak strength (ε_{co}) is determined by the expression given by Tasdemir et al. [44] (Eq. 12).

$$\varepsilon_{cu} = c_2 \varepsilon_{co} + k_2 \left(\frac{K_l}{f'_{co}} \right)^{0.9} \varepsilon_{h,rupt}^{1.35} \quad \text{Eq. 10}$$

$$c_2 = 2 - \frac{(f'_{co} - 20)}{100} \text{ and } c_2 \geq 1 \quad \text{Eq. 11}$$

$$\varepsilon_{co} = \left(-0.067 f'_{co}{}^2 + 29.9 f'_{co} + 1053 \right) \times 10^{-6} \quad \text{Eq. 12}$$

In Eqs. 11 and 12, f'_{co} is in MPa.

Table 10 summarizes the values of the strain enhancement coefficient (k_2) values calibrated from the database for different types of FRP materials, confinement methods and axial strain measurement methods. As discussed previously in Section 3.5, the magnitude of the recorded ultimate axial strains may be influenced by the methods used in the measurement of the strains. In Table 10, in addition to the average values of the strain enhancement coefficients (k_2), its specific values obtained from each of the three aforementioned axial strain measurement methods are also given. As can be seen in Table 10, k_2 is not sensitive to FRP type and hence it is recommended that an average value of $k_2 = 0.27$ can be used in Eq.10 independent of FRP material type.

4.4. Comparison with test data

Figure 6 shows comparisons of the strength and strain enhancement (f'_{cc}/f'_{co} and $\varepsilon_{cu}/\varepsilon_{co}$) predictions of the proposed model with results from the database presented in this paper. These comparisons indicate that the model predictions are in close agreement with the test results, which are quantified through the use of statistical indicators: average absolute error (*AAE*) to establish overall model accuracy; mean (*M*) to establish average overestimation or underestimation of the model; and standard deviation (*SD*) to establish the magnitude of the associated scatter. The details of the assessment procedure can be found in Ozbakkaloglu et al. [1].

To establish the relative performance of the proposed model, its prediction statistics were compared with those of a group of selected models, which were identified as the best performing models [3-5, 7, 32, 42, 43, 45-53] among over 80 existing models reviewed in Ozbakkaloglu et al. [1] and a few additional models proposed in 2012 and 2013. The lists of the 10 most accurate strength and strain models are given in Tables 11 and 12, respectively, together with their prediction statistics for strength and strain enhancement ratios (f'_{cc}/f'_{co} and $\varepsilon_{cu}/\varepsilon_{co}$). Figures 7(a) and 7(b), respectively, show the average absolute errors (*AAE*) of the strength and strain enhancement ratio predictions of these models. The comparisons of the model prediction statistics shown in Figure 7 and Tables 11 and 12 demonstrate the improved accuracy of the proposed model over the best performing existing models. The improvement on the prediction of the ultimate strain enhancement ratio ($\varepsilon_{cu}/\varepsilon_{co}$) is particularly significant, which is achieved through the use of an expression (Eq.10) that accurately captures the relative influences of the key parameters. It might be worth noting that in the evaluation of the models, the experimentally recorded hoop rupture strains ($\varepsilon_{h,rupt}$) were used rather than the values or expressions recommended by the original models for the calculation of $\varepsilon_{h,rupt}$. In the absence of the experimental values, $\varepsilon_{h,rupt}$ was established using the average value of $k_{\varepsilon,f}$ or $k_{\varepsilon,frp}$ reported in Table 8 in the assessment of the existing models, and it was calculated from Eq.4 in the assessment of the proposed model. It might be worth noting that the proposed model would have

outperformed the existing models even more significantly if the hoop rupture strains were established using the original model expressions.

5. CONCLUSIONS AND RECOMMENDATIONS

This paper has presented a comprehensive test database of 832 datasets that was assembled by the authors through an extensive review of the literature that covered 253 experimental studies published on the compressive behavior of FRP-confined concrete. Initially, 3042 test results were collected from the published literature. The suitability of these results for the database was then assessed using carefully composed selection criteria to ensure the reliability and consistency of the database. Using the criteria to refine the contents of the database resulted in a final database size of 832 datasets collected from 99 experimental studies published between 1992 and the middle of 2013. Key features of each study included in the database, including the range of the key test parameters and the specimen instrumentation information, have been summarized and important observations regarding these studies have been marked on the database tables. The database that has been presented in this paper will serve as a valuable reference document for future model development efforts. In the final part of the paper, a design-oriented model for predicting the ultimate conditions of FRP-confined concrete is presented. The proposed model provides improved predictions of the compressive strength and ultimate axial strain of FRP-confined concrete compared to the existing models.

Based on the observations made during the compilation of the experimental database, the following conclusions can be drawn:

1. Analysis of the results reported in the database indicates that the average values of the hoop strain reduction factors based on fiber and FRP properties ($k_{\epsilon,f}$ and $k_{\epsilon,frp}$) are equal to 0.675 and 0.709, respectively. The observed variation of the average k_{ϵ} values according to fiber type points to the possible influence of the type of fibers on the strain reduction factor.

2. Two key ultimate condition properties, namely ultimate axial strain (ε_{cu}) and hoop rupture strain ($\varepsilon_{h,rupt}$), are both highly sensitive to the instrumentation arrangement used in specimen testing. Therefore, the variability in the instrumentation arrangements used in different studies contributes to scatter in the database.
3. There are differences between the strength and strain enhancement ratios of FRP-wrapped and FRP-tube encased specimens included in the database of the present study. However, due to the differences between the number and parametric ranges of FRP-wrapped and FRP tube-encased specimens, it is not possible to draw a definitive conclusion based on these observations.

As noted previously, it was not possible to include all the test results published in the literature in the database presented in this paper, due to a lack of information in regards to the material properties, geometric properties or ultimate conditions of these specimens. Therefore, in future studies, effort should be made to ensure that the results of the experiments are presented with a complete set of information, providing as much relevant information as possible about the material and geometric properties of the specimens, test setup and instrumentation, recorded capacities of the specimens and their failure modes. Furthermore, in future experimental studies due consideration should be given to the instrumentation of the specimens for the accurate measurement of ultimate axial strains and hoop rupture strains.

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7. NOTATION

AAE	Average absolute error
c_1	Parameter in ultimate strength expression
c_2	Parameter in ultimate strain expression
D	Diameter of concrete core (mm)
E_f	Elastic modulus of fibers (MPa)
E_{frp}	Elastic modulus of FRP material (MPa)
f'_{cc}	Ultimate axial compressive stress of FRP-confined concrete (MPa)
f'_{co}	Peak axial compressive stress of unconfined concrete (MPa)
f'_{c1}	Axial compressive stress of FRP-confined concrete at first peak (MPa)
f_f	Ultimate tensile strength of fibers; $f_f = E_f \varepsilon_f$ (MPa)
f_{frp}	Ultimate tensile strength of FRP material; $f_{frp} = E_{frp} \varepsilon_{frp}$ (MPa)
f_l	Confining pressure (MPa)
f_{lo}	Threshold confining pressure (MPa)
f_{lu}	Nominal lateral confining pressure at ultimate; $f_{lu} = K_l \varepsilon_f$ or $f_{lu} = K_l \varepsilon_{frp}$ (MPa)
$f_{lu,a}$	Actual lateral confining pressure at ultimate; $f_{lu,a} = K_l \varepsilon_{h,rupt}$ (MPa)
H	FRP confined concrete specimen height (mm)
K_l	Lateral confinement stiffness (MPa); $K_l = 2 E_f t_f / D$ or $2 E_{frp} t_{frp} / D$
k_1	Axial strength enhancement coefficient
k_2	Axial strain enhancement coefficient
k_ε	Hoop strain reduction factor
$k_{\varepsilon,f}$	Hoop strain reduction factor of fibers
$k_{\varepsilon,frp}$	Hoop strain reduction factor of FRP material
M	Mean
SD	Standard deviation
t_f	Total nominal thickness of fibers (mm)
t_{frp}	Total thickness of FRP material (mm)
ε_{co}	Axial strain of unconfined concrete at f'_{co}
ε_{c1}	Axial strain of FRP-confined concrete at f'_{c1}
ε_{cu}	Ultimate axial strain of FRP-confined concrete
ε_f	Ultimate tensile strain of fibers
ε_{frp}	Ultimate tensile strain of FRP material
$\varepsilon_{h,rupt}$	Hoop rupture strain of FRP shell
ε_{l1}	Hoop strain of FRP-confined concrete at f'_{c1}

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Table 1. Summary of existing databases of axial compression tests on circular FRP-confined concrete specimens

Paper	Number of studies covered	Number of test data	Confinement technique	Fiber type	Unconfined concrete strength range, f_{co} (MPa)	Nominal confinement ratio range, f_{lu}/f_{co}	Specimen Diameter range, D (mm)	Specimen Height range, H (mm)
Lam and Teng [2, 3]	30	275	Tube and wrap	CFRP, GFRP, AFRP, and HM CFRP	18.0 - 62.4	0.03 - 2.30	51 - 200	102 - 788
De Lorenzis and Tepfers [4]	17	180	Tube and wrap	CFRP, GFRP, and AFRP	19.4 - 82.1	0.06 - 2.31	51 - 219	102 - 438
Bisby et al. [5]	23	197	Wrap	CFRP, GFRP, and AFRP	15 - 103	-	50 - 300	-
Turgay et al. [6]	20	127	Tube and wrap	CFRP	17.4 - 171.0	0.032 - 0.95	51 - 200	102 - 610
Realfonzo and Napoli [7]	63	465	Wrap	CFRP, GFRP	15.2 - 169.7	0.002 - 2.22	51 - 406	102 - 1824

Table 2. Summary of test results included in the database

Paper	Total number of datasets	Confinement technique	Fiber type	Diameter, D (mm)	Height, H (mm)	Unconfined concrete strength range, f_{co} (MPa)	Strength enhancement ratio range, f_{cc}/f_{co}	Strain enhancement ratio range, $\epsilon_{cu}/\epsilon_{co}$	Actual confinement ratio range, $f_{lu,a}/f_{co}$	Number of identical specimens per entry in the database	Lateral strain measurement method (see notes)	Axial strain measurement method (see notes)
Abdollahi et al. [54]	5	Wrap	GFRP	150	300	14.8 - 41.7	1.24 - 3.32	1.54 - 12.04	0.06 - 0.43	2	N/A	AFL
Ahmad et al. [55]	2	Wrap	GFRP	102	203	39.0 - 50.5	2.68 - 2.96	-	0.55 - 0.73	Single	N/A	N/A
Aire et al. [56]	6	Wrap	CFRP, GFRP	150	300	42.0	0.97 - 2.57	3.33 - 13.17	0.09 - 0.71	3	HS	AFL
Akogbe et al. [57]	12	Wrap	CFRP	100 - 300	200 - 600	21.7 - 26.5	2.38 - 3.19	7.03 - 12.68	0.26 - 0.32	Single	HS	AML
Almusallam [58]	4	Wrap	GFRP	150	300	47.7 - 50.8	1.09 - 2.10	-	0.14 - 0.46	3	HL	N/A
Al-Salloum [45]	2	Wrap	CFRP	150	300	32.4 - 36.2	2.35 - 2.57	15.77	0.30 - 0.33	Single	HS	AML
Au and Buyukozturk [59]	1	Wrap	GFRP	150	375	24.2	1.81	6.19	0.26	3	HL	AML
Benzaid et al. [60]	4	Wrap	CFRP	160	320	25.9 - 49.5	1.07 - 2.55	1.48 - 5.57	0.09 - 0.59	Single	HL	AML
Berthet et al. [61]	42	Wrap	CFRP, GFRP	160	320	25.0 - 52.0	1.12 - 4.15	2.18 - 13.50	0.07 - 0.95	Single	HL	AML
Bisby et al. [62]	3	Wrap	CFRP	150	300	34.4	1.25 - 1.28	2.42 - 2.73	0.10 - 0.13	Single	N/A	N/A
Bisby et al. [63]	3	Wrap	CFRP	100	200	28.0	1.89 - 2.25	4.24 - 5.28	0.20 - 0.22	Single	N/A	N/A
Bullo [64]	12	Wrap	GFRP, HM CFRP	150	300	32.5	1.62 - 4.17	3.36 - 19.53	0.11 - 0.60	Single	HL	AFL
Campione et al. [65]	1	Wrap	CFRP	100	200	20.1	2.47	12.32	0.36	N/A	N/A	N/A
Carey and Harries [66]	2	Wrap	CFRP	152 - 254	305 - 762	33.5 - 38.9	1.40 - 1.41	3.47 - 4.04	0.15 - 0.17	≥ 2	HL	AML
Comert et al. [67]	2	Wrap	GFRP	150	300	39.0	1.56 - 1.64	9.92 - 10.86	0.23	Single	HS	AFL
Cui and Sheikh [68]	24	Wrap	CFRP, GFRP, HM CFRP	152	305	45.6 - 48.1	1.21 - 3.38	4.24 - 13.92	0.07 - 0.48	Single	HS	AML
Dai et al. [40]	9	Wrap	AFRP	152	305	39.2	1.42 - 3.01	9.75 - 22.52	0.09 - 0.39			
Demers and Neale [69]	8	Wrap	CFRP, GFRP	152	305	32.2 - 43.7	0.96 - 1.72	4.35 - 10.48	0.07 - 0.24	Single	HS	AFL

Elsanadedy et al. [70]	6	Wrap	CFRP	50 - 150	100 - 300	41.1 - 53.8	1.86 - 3.51	2.61 - 4.54	0.20 - 0.59	2 to 5	N/A	AML
Erdil et al. [71]	2	Wrap	CFRP	150	300	11.1 - 20.8	2.28 - 2.96	11.67 - 14.00	0.23 - 0.44	3	HS	AML
Evans et al. [72]	1	Wrap	CFRP	150	300	37.3	1.73	6.31	0.28	Single	HS	AFL
Green et al. [73]	3	Wrap	CFRP, GFRP	152	305	46.0 - 54.0	1.15 - 1.28	-	0.05 - 0.10	Single	HS	N/A
Harmon and Slattery [74]	4	Wrap	CFRP	51	102	41.0	2.10 - 5.88	5.08 - 15.70	0.19 - 1.42	Single	HS	AFL
Harries and Carey [8]	4	Wrap, unbonded wrap	GFRP	152	305	31.8	1.06 - 1.52	2.32	0.08 - 0.21	≥ 5	HL	AML
Harries and Kharel [75]	10	Wrap	CFRP, GFRP	152	305	32.1	1.02 - 1.87	1.43 - 4.93	0.02 - 0.33	≥ 5	HL	AML
Hong and Kim [13]	2	Tube	CFRP	300	600	17.5	4.32 - 4.58	14.33 - 18.51	1.11	Single	HS	AML
Hosotani et al. [76]	2	Wrap	CFRP, HM CFRP	200	600	41.7	2.16 - 2.23	4.41 - 6.18	0.23 - 0.25	Single	N/A	N/A
Howie and Karbhari [77]	12	Wrap	CFRP	152	305	38.6	1.09 - 2.33	-	0.06 - 0.40	Single	HL	N/A
Ilki et al. [78]	5	Wrap	CFRP	150	300	32.0	1.48 - 3.37	7.20 - 24.80	0.12 - 0.79	Single	HS	AFL
Ilki et al. [79]	12	Wrap	CFRP	150	300	6.2	3.13 - 17.47	13.00 - 52.00	0.55 - 4.74	Single	HS	AFL
Issa [80]	3	Wrap	CFRP	150	300	23.6 - 23.9	1.66 - 1.77	-	0.17 - 0.18	Single	HL	N/A
Issa and Karam [81]	9	Wrap	CFRP	150	300	30.5	1.17 - 2.48	-	0.14 - 0.41	Single	HL	N/A
Jiang and Teng [32]	23	Wrap	CFRP, GFRP	152	305	33.1 - 47.6	0.88 - 4.24	2.66 - 17.05	0.06 - 0.99	Single	HS	AML
Karabinis and Rousakis [82]	16	Wrap	CFRP	200	320	35.7 - 38.5	1.08 - 1.89	1.26 - 8.99	0.07 - 0.23	Single	N/A	AML
Karam and Tabbara [83]	2	Wrap	CFRP	150	300	12.8	1.39 - 2.48	2.91 - 5.91	0.29 - 0.59	2	HL	AML
Karantzakis et al. [84]	2	Wrap, unbonded wrap	CFRP	200	350	12.1	1.78 - 2.42	5.27 - 8.73	0.22	3	N/A	AML
Karbhari and Gao [37]	3	Wrap	CFRP	152	305	38.4	1.56 - 2.33	6.18 - 11.42	0.25 - 0.41	≥ 3	HL	AFL
Kono et al. [85]	15	Wrap	CFRP	100	200	32.3 - 34.8	1.46 - 3.16	3.93 - 12.37	0.14 - 0.62	Single	N/A	N/A
Lam and Teng [18]	18	Wrap	CFRP, GFRP	152	305	34.3 - 38.5	1.31 - 2.84	5.44 - 13.38	0.13 - 0.42	Single	HS	AML
Lam et al. [29]	6	Wrap	CFRP	152 - 152.5	304 - 305	38.9 - 41.1	1.28 - 2.03	3.52 - 8.32	0.11 - 0.31	Single	HS	AML
Lee et al. [86]	5	Wrap	CFRP	150	300	36.2	1.15 - 2.88	4.17 - 12.92	0.11 - 0.56	Single	HL	AML
Li et al. [87]	1	Wrap	GFRP	152.4	305	45.6	1.08	-	0.24	3	N/A	N/A
Li et al. [88]	2	Tube	GFRP	150	300	47.5	1.07 - 1.80	2.25 - 5.25	0.09 - 0.15	N/A	N/A	N/A
Liang et al. [89]	12	Wrap	CFRP	100	200	22.7 - 25.9	2.4 - 3.04	7.78 - 12.27	0.29 - 0.44	Single	HL	AFL
Lin and Chen [38]	10	Wrap	GFRP, HM CFRP	120	240	32.7	1.52 - 3.20	-	0.10 - 0.55	Single	N/A	N/A
Lin and Li [90]	27	Wrap	CFRP	100 - 150	200 - 300	17.7 - 25.9	1.92 - 5.23	-	0.19 - 1.23	3	HS	N/A
Lin and Liao [91]	6	Wrap	CFRP	100	200	23.9	2.57 - 3.91	-	0.51 - 0.96	Single	N/A	N/A
Mandal et al. [92]	9	Wrap	CFRP, GFRP	102 - 105	200	30.7 - 54.5	1.17 - 2.58	1.33 - 11.41	0.16 - 0.74	3	N/A	AML
Mastrapa [10]	13	Wrap, unbonded wrap	GFRP	152.5	305	29.8 - 37.2	0.90 - 3.10	7.96 - 32.54	0.19 - 1.03	Single	HS	AFL
Matthys et al. [11]	4	Wrap, unbonded wrap	CFRP, UHM CFRP	150	300	34.9	1.17 - 1.27	1.71 - 4.22	0.10 - 0.12	Single	HS	AS
Micelli et al. [93]	2	Wrap	CFRP, GFRP	102	204	32.0 - 37.0	1.61 - 1.62	4.93 - 8.93	0.19 - 0.23	N/A	N/A	N/A
Mirmiran et al. [9]	26	Wrap, unbonded wrap	GFRP	152.5	305	29.8 - 31.2	1.04 - 3.24	5.31 - 32.80	0.15 - 0.78	Single	HS	AFL
Miyauchi et al. [94]	10	Wrap	CFRP	100 - 150	200 - 300	31.2 - 51.9	1.31 - 3.26	4.32 - 10.32	0.07 - 0.42	2	N/A	AS
Miyauchi et al. [95]	6	Wrap	CFRP	100 - 150	200 - 300	23.6 - 26.3	1.55 - 3.23	8.83 - 13.24	0.14 - 0.55	N/A	N/A	N/A
Modarelli et al. [96]	3	Wrap	CFRP, GFRP	150	300	28.4 - 38.2	1.64 - 1.95	2.37 - 4.49	0.13 - 0.26	3	HS	AFL
Nanni and Bradford [97]	17	Wrap	GFRP, AFRP	150	300	35.6 - 36.3	1.13 - 5.40	9.21 - 47.37	0.18 - 1.66	Single	N/A	AFL
Ongpeng [98]	2	Wrap	CFRP	180	500	27.0	1.38 - 1.90	-	0.12 - 0.25	Single	N/A	N/A
Owen [99]	8	Wrap	CFRP	102 - 152	203 - 305	47.9 - 53.0	1.33 - 4.89	3.86 - 17.02	0.15 - 1.66	1 to 4	N/A	N/A
Ozbakkaloglu and Akin [39]	4	Wrap	AFRP	152	305	39.0	1.72 - 2.25	10.95 - 14.80	0.25 - 0.45	Single	HS	AFL
Ozbakkaloglu and Vincent [14]	24	Tube	CFRP, AFRP, UHM CFRP	74 - 302	152 - 600	34.0 - 55.0	1.06 - 2.47	3.29 - 15.97	0.05 - 0.38	Single	HS	AFL
Park et al. [100]	12	Tube	GFRP	150	300 - 450	32.0 - 54.0	1.69 - 3.82	7.73 - 15.69	0.11 - 0.58	Single	N/A	AFL
Picher et al. [101]	1	Wrap	CFRP	152	304	39.7	1.41	5.01	0.21	3	HL	AFL
Piekarczyk et al. [102]	2	Wrap	CFRP	47	112	55.0	2.18 - 3.44	2.86 - 4.00	0.52 - 0.86	Single	N/A	N/A
Pon et al. [103]	8	Wrap	CFRP	150 - 600	300 - 1200	7.1 - 9.6	1.73 - 4.68	-	0.28 - 1.30	N/A	N/A	N/A
Rochette and Labossière [104]	7	Wrap	CFRP, AFRP	100 - 150	200 - 300	42.0 - 43.0	1.10 - 1.75	5.01 - 7.86	0.08 - 0.26		HL	AFL
Rousakis [105]	20	Wrap	HM CFRP	150	300	25.2 - 51.8	1.36 - 2.67	2.22 - 7.88	0.07 - 0.46	Single	HS	AFL
Rousakis et al. [26]	6	Wrap	CFRP	150	300	20.4 - 49.2	1.61 - 3.09	2.06 - 5.46	0.13 - 0.95	Single	HS	AML

Saafi et al. [12]	6	Tube	GFRP, HM CFRP	152	435	35.0	1.51 - 2.77	4.00 - 12.00	0.07 - 0.40	3	HS	AFL
Saenz and Pantelides [106]	4	Wrap	CFRP	152	304	40.3 - 47.5	1.72 - 2.68	3.79 - 9.49	0.22 - 0.59	3	HS	AML
Santarosa et al. [107]	3	Wrap	CFRP	150	300	15.3 - 28.1	1.37 - 3.05	3.01 - 8.70	0.11 - 0.42	2	HS	AS
Shahawy et al. [16]	9	Wrap	CFRP	152.5	305	19.4 - 49.0	1.21 - 4.13	2.14 - 10.79	0.30 - 4.00	5	HS	AML
Shao et al. [108]	2	Wrap	GFRP	152	305	40.2	1.23 - 1.78	-	0.18 - 0.37	Single	HS	N/A
Shehata et al. [109]	4	Wrap	CFRP	150	300	25.6 - 29.8	1.71 - 2.42	5.86 - 8.29	0.19 - 0.41	9	N/A	N/A
Shehata et al. [110]	4	Wrap	CFRP	150 - 225	300 - 450	34.0	1.29 - 2.41	3.10 - 5.50	0.10 - 0.29	9	N/A	N/A
Silva and Rodrigues [111]	7	Wrap	GFRP	150 - 250	300 - 750	29.6 - 31.2	1.79 - 3.03	4.54 - 11.33	0.20 - 0.58	Single	HS	N/A
Smith et al. [36]	4	Wrap	CFRP	250	500	35.0	1.43 - 1.69	-	0.11 - 0.17	Single	HS	N/A
Song et al. [112]	12	Wrap	CFRP	100 - 150	300 - 450	22.4	1.40 - 5.30	4.01 - 19.61	0.12 - 0.88	2	HS	AFL
Stanton and Owen [30]	5	Wrap	CFRP	152.5	305	49.0	1.41 - 5.63	4.24 - 19.51	0.11 - 0.90	N/A	N/A	N/A
Suter and Pinzelli [113]	16	Wrap	CFRP, GFRP, AFRP, UHM CFRP	150	300	33.3 - 54.0	1.14 - 3.12	1.16 - 8.92	0.09 - 0.46	≥ 1	N/A	N/A
Tamuzs et al. [114]	4	Wrap	CFRP	150	300	20.8 - 48.8	1.48 - 2.03	3.21 - 5.48	0.25 - 0.62	Single	HS	AS
Teng et al. [115]	6	Wrap	GFRP	152.5	305	39.6	0.94 - 1.66	3.14 - 9.73	0.07 - 0.26	Single	HS	AML
Thériault et al. [116]	5	Wrap	CFRP, GFRP	51 - 304	102 - 608	18.0 - 37.0	1.73 - 3.89	-	0.25 - 1.15	3	N/A	N/A
Valdmanis et al. [27]	6	Wrap	CFRP	150	300	40.0 - 44.3	1.65 - 2.60	3.18 - 8.00	0.09 - 0.28	Single	HS	AML
Vincent and Ozbakkaloglu [117]	6	Wrap	CFRP	152	305	35.5 - 38.0	1.21 - 1.74	3.79 - 7.88	0.11 - 0.23	Single	HS	AFL
Vincent and Ozbakkaloglu [118]	12	Wrap, tube	AFRP	152	305	49.4	2.09 - 2.24	12.59 - 15.76	0.30 - 0.42	Single	HS	AFL
Wang and Wu [33]	12	Wrap	CFRP	150	300	30.9 - 52.1	1.28 - 2.85	-	0.09 - 0.43	Single	HS	N/A
Wang and Wu [119]	18	Wrap	AFRP	70 - 194	210 - 582	24.0 - 51.6	0.98 - 3.37	1.36 - 5.68	0.04 - 0.35	N/A	N/A	N/A
Wang and Zhang [120]	2	Wrap	AFRP	150	450	47.3 - 51.1	1.73 - 1.78	6.01 - 6.99	0.20 - 0.22	Single	N/A	AS
Watanabe et al. [28]	9	Wrap	CFRP, AFRP, UHM CFRP	100	200	30.2	1.29 - 3.46	2.48 - 24.13	0.14 - 0.79	N/A	N/A	N/A
Wong et al. [34]	4	Wrap	GFRP	152.5	305	36.5 - 46.7	1.24 - 1.73	5.58 - 8.40	0.14 - 0.27	Single	HS	AML
Wu and Jiang [121]	4	Wrap	CFRP	150	300	28.7 - 30.1	1.91 - 3.00	-	0.17 - 0.32	Single	N/A	AML
Wu and Jiang [24]	34	Wrap	CFRP	150	300	20.6 - 36.7	1.69 - 6.83	-	0.15 - 1.31	Single	HS	AML
Wu et al. [31]	4	Wrap	CFRP, AFRP, HM CFRP, GFRP	150	300	23.0	1.96 - 2.30	-	0.15 - 0.23	Single	HS	N/A
Wu et al. [35]	10	Wrap	CFRP, GFRP, AFRP, UHM CFRP	150	300	23.1	1.94 - 3.55	4.49 - 14.04	0.15 - 0.42	Single	HS	AFL
Wu et al. [122]	2	Wrap	AFRP	100	300	46.4	1.69 - 2.77	3.54 - 7.37	0.17 - 0.34	Single	N/A	AML
Xiao and Wu [15]	27	Wrap	CFRP	152	305	33.7 - 55.2	1.05 - 2.83	1.66 - 15.27	0.06 - 0.51	Single	HS	AS
Yan et al. [123]	1	Wrap	CFRP	305	610	15.2	2.49	5.50	0.39	Single	HS	AML
Youssef et al. [53]	40	Wrap	CFRP, GFRP	152.4 - 406.4	304.8 - 812.8	29.4 - 44.6	1.44 - 4.31	2.56 - 14.24	0.10 - 0.88	Single	HS	AML
Zhang et al. [124]	1	Wrap	CFRP	150	300	34.3	1.73	10.50	0.30	5	N/A	AFL

Specimen instrumentation notes:

HS denotes hoop strains were measured by strain gauges attached on the surface of specimens

HL denotes hoop strains were measured by lateral LVDTs, extensometers, or dial gauges mounted on specimens

AS denotes axial strains were measured by strain gauges attached on the surface of specimens

AFL denotes axial strains were determined from LVDTs or dial gauges mounted on loading platens to measure deformations along the full height of specimens

AML denotes axial strains were determined from LVDTs, extensometers, or dial gauges mounted on specimens to measure deformations within a gauge length along the height of specimens

N/A denotes information that was either not applicable to the dataset or not available in the source document

Xiao and Wu [15]	152	305	55.2		105	1577	1.143				103.3	1.18	0.70	0.466 [^]	
Yan et al. [123]	305	610	15.0	0.200	86.9	1220	1				37.8	1.1			
Youssef et al. [53]	406.4	812.8	29.4	0.24	103.84	1246	5.840				125.80	2.813			
Youssef et al. [53]	406.4	812.8	29.4	0.24	103.84	1246	5.840				126.39	2.914			
Youssef et al. [53]	406.4	812.8	29.4	0.24	103.84	1246	5.840				127.01	2.801			
Youssef et al. [53]	406.4	812.8	29.4	0.24	103.84	1246	3.504				83.05	1.492			
Youssef et al. [53]	406.4	812.8	29.4	0.24	103.84	1246	3.504				88.68	1.621			
Youssef et al. [53]	406.4	812.8	29.4	0.24	103.84	1246	2.336				64.78	1.155			
Youssef et al. [53]	406.4	812.8	29.4	0.24	103.84	1246	2.336				62.09	1.112			
Youssef et al. [53]	406.4	812.8	29.4	0.24	103.84	1246	2.336				67.47	1.199			
Youssef et al. [53]	406.4	812.8	29.4	0.24	103.84	1246	1.168				45.95	0.647			
Youssef et al. [53]	406.4	812.8	29.4	0.24	103.84	1246	1.168				45.78	0.615			
Youssef et al. [53]	152.4	304.8	44.6	0.20	103.84	1246	2.336				124.08	2.847			
Youssef et al. [53]	152.4	304.8	44.6	0.20	103.84	1246	2.336				129.17	2.792			
Youssef et al. [53]	152.4	304.8	44.6	0.20	103.84	1246	2.336				138.72	2.844			
Youssef et al. [53]	152.4	304.8	44.6	0.20	103.84	1246	1.752				94.24	1.996			
Youssef et al. [53]	152.4	304.8	44.6	0.20	103.84	1246	1.752				95.02	1.999			
Youssef et al. [53]	152.4	304.8	44.6	0.20	103.84	1246	1.752				100.52	1.979			
Youssef et al. [53]	152.4	304.8	44.6	0.20	103.84	1246	1.168				85.96	1.706			
Youssef et al. [53]	152.4	304.8	44.6	0.20	103.84	1246	1.168				88.14	2.003			
Youssef et al. [53]	152.4	304.8	44.6	0.20	103.84	1246	1.168				84.23	1.996			
Zhang et al. [124]	150	300	34.3		91 ^{p,m}	753 ^{p,m}	1	240	3800	0.33	59.4	2.1			

- p* denotes fiber tensile strength and elastic modulus are given in N/mm-ply
t denotes FRP properties calculated based on total nominal ply thickness of fiber sheet
m denotes FRP material properties that differ significantly from the reference properties of the corresponding material
d denotes ultimate axial stress values that are lower than the unconfined concrete strength
s denotes inconsistent axial stress when compared with overall trend in the database
a denotes inconsistent axial strain when compared with overall trend in the database
[^] denotes inconsistent k_c values when compared with overall trend in the database

Table 5. Test database of AFRP -wrapped concrete specimens

Paper	Specimen Dimensions		Concrete Properties		FRP Properties			Fiber Properties			Measured Ultimate Conditions			Hoop Rupture Strain Reduction Factors	
	D (mm)	H (mm)	f'_{co} (MPa)	ε_{co} (%)	E_{frp} (GPa)	f_{frp} (MPa)	t_{frp} (mm)	E_f (GPa)	f_f (MPa)	t_f (mm)	f'_{cc} (MPa)	ε_{cu} (%)	$\varepsilon_{h,rupt}$ (%)	$k_{\varepsilon,frp}$	$k_{\varepsilon,f}$
Dai et al. [40]	152	305	39.2		115.2 ^t	3732 ^t		78 ^t	2400 ^t	0.169	61.4	2.33	3.16	0.975	1.053 [^]
Dai et al. [40]	152	305	39.2		115.2 ^t	3732 ^t		78 ^t	2400 ^t	0.169	62.7	2.33	3.13	0.966	1.043 [^]
Dai et al. [40]	152	305	39.2		115.2 ^t	3732 ^t		78 ^t	2400 ^t	0.169	55.8	2.07	3.21	0.991	1.070 [^]
Dai et al. [40]	152	305	39.2		115.2 ^t	3732 ^t		78 ^t	2400 ^t	0.338	90.1	3.80 ^a	2.89	0.892	0.963
Dai et al. [40]	152	305	39.2		115.2 ^t	3732 ^t		78 ^t	2400 ^t	0.338	88.3	3.45	3.05	0.941	1.017 [^]
Dai et al. [40]	152	305	39.2		115.2 ^t	3732 ^t		78 ^t	2400 ^t	0.338	83.3	3.68 ^a	2.96	0.914	0.987
Dai et al. [40]	152	305	39.2		115.2 ^t	3732 ^t		78 ^t	2400 ^t	0.507	113.2	4.39	2.74	0.846	0.913
Dai et al. [40]	152	305	39.2		115.2 ^t	3732 ^t		78 ^t	2400 ^t	0.507	116.3	4.6 ^a	2.46	0.759	0.820
Dai et al. [40]	152	305	39.2		115.2 ^t	3732 ^t		78 ^t	2400 ^t	0.507	118	4.78	2.97	0.917	0.990
Nanni and Bradford [97]	150	300	35.6		62.2	1150	3.8	127.5	2640	2.16	192.21 ^s	9.628 ^a			
Nanni and Bradford [97]	150	300	35.6		62.2	1150	3.8	127.5	2640	2.16	186.35 ^s	6.778 ^a			
Ozbakkaloglu and Akin [39]	152	305	39					120	2900	0.4	69.2	2.32	1.71		0.684
Ozbakkaloglu and Akin [39]	152	305	39					120	2900	0.4	67.1	2.30	1.56		0.624
Ozbakkaloglu and Akin [39]	152	305	39					120	2900	0.6	87.6	3.11	1.84		0.736
Ozbakkaloglu and Akin [39]	152	305	39					120	2900	0.6	85.0	2.86	1.66		0.664
Rochette and Labossière [104]	150	300	43		13.6	230	1.27				47.3	1.11	1.55	0.917	
Rochette and Labossière [104]	150	300	43		13.6	230	2.56				58.9	1.47	1.39	0.822	
Rochette and Labossière [104]	150	300	43		13.6	230	3.86				71.0	1.69	1.33	0.786	
Rochette and Labossière [104]	150	300	43		13.6	230	5.21				74.4	1.74	1.18	0.698	
Suter and Pinzelli [113]	150	300	44.7		31.2	602.2	0.7	120	2900	0.193	52.23	0.238 ^a			
Suter and Pinzelli [113]	150	300	44.7		31.2	602.2	1.4	120	2900	0.386	76.85	1.136			
Suter and Pinzelli [113]	150	300	44.7		31.2	602.2	2.1	120	2900	0.579	103.45	1.300			
Suter and Pinzelli [113]	150	300	44.7		31.2	602.2	2.8	120	2900	0.772	136.89	1.784			
Suter and Pinzelli [113]	150	300	36.2		31.2	602.2	0.7	120	2900	0.193	48.15	0.664			
Suter and Pinzelli [113]	150	300	36.2		31.2	602.2	1.4	120	2900	0.386	75.30	1.006			
Suter and Pinzelli [113]	150	300	36.2		31.2	602.2	2.1	120	2900	0.579	98.46	1.304			
Suter and Pinzelli [113]	150	300	33.3		31.2	602.2	0.7	120	2900	0.193	50.28	0.790			
Suter and Pinzelli [113]	150	300	33.3		31.2	602.2	1.4	120	2900	0.386	78.59	1.302			
Suter and Pinzelli [113]	150	300	33.3		31.2	602.2	2.1	120	2900	0.579	103.90	1.502			
Suter and Pinzelli [113]	150	300	54		31.2	602.2	0.7	120	2900	0.193	61.56	0.342 ^a			
Suter and Pinzelli [113]	150	300	54		31.2	602.2	1.4	120	2900	0.386	84.24	0.638 ^a			
Suter and Pinzelli [113]	150	300	54		31.2	602.2	2.1	120	2900	0.579	111.24	0.816 ^a			
Vincent and Ozbakkaloglu [118]	152	305	49.4					120	2900	0.6	109.0	3.73	2.54		1.016 [^]
Vincent and Ozbakkaloglu [118]	152	305	49.4					120	2900	0.6	103.4	3.40	2.10		0.839
Vincent and Ozbakkaloglu [118]	152	305	49.4					120	2900	0.6	105.3	3.37	2.08		0.831
Vincent and Ozbakkaloglu [118]	152	305	49.4					120	2900	0.6	107.7	3.41	2.18		0.873
Vincent and Ozbakkaloglu [118]	152	305	49.4					120	2900	0.6	104.0	3.22	2.12		0.848
Vincent and Ozbakkaloglu [118]	152	305	49.4					120	2900	0.6	110.1	3.48	2.22		0.888
Wang and Wu [119]	70	210	51.63	0.248				118	2060	0.057	65.97	0.403 ^a			
Wang and Wu [119]	70	210	51.63	0.248				118	2060	0.095	72.63	0.530 ^a			
Wang and Wu [119]	70	210	51.63	0.248				118	2060	0.191	111.43	0.567 ^a			
Wang and Wu [119]	105	315	50.64	0.244				118	2060	0.072	59.48	0.331 ^a			
Wang and Wu [119]	105	315	50.64	0.244				118	2060	0.143	62.69	0.387 ^a			
Wang and Wu [119]	105	315	50.64	0.244				118	2060	0.286	96.02	0.423 ^a			
Wang and Wu [119]	194	582	44.92	0.260				118	2060	0.143	44.00	0.358 ^a			
Wang and Wu [119]	194	582	44.92	0.260				118	2060	0.286	58.75	0.387 ^a			
Wang and Wu [119]	194	582	44.92	0.260				118	2060	0.572	106.03	0.460 ^a			
Wang and Wu [119]	70	210	29.37	0.203				118	2060	0.095	49.64	0.537 ^a			
Wang and Wu [119]	70	210	29.37	0.203				118	2060	0.057	41.80	0.360 ^a			
Wang and Wu [119]	70	210	29.37	0.203				118	2060	0.191	86.07 ^s	0.953 ^a			
Wang and Wu [119]	105	315	28.79	0.202				118	2060	0.072	41.20	0.363 ^a			
Wang and Wu [119]	105	315	28.79	0.202				118	2060	0.143	47.77	0.583 ^a			
Wang and Wu [119]	105	315	28.79	0.202				118	2060	0.286	87.42 ^s	1.147			
Wang and Wu [119]	194	582	23.98	0.207				118	2060	0.143	33.84	0.383 ^a			
Wang and Wu [119]	194	582	23.98	0.207				118	2060	0.286	43.90	0.513 ^a			
Wang and Wu [119]	194	582	23.98	0.207				118	2060	0.572	80.86 ^s	0.933 ^a			
Wang and Zhang [120]	150	450	47.3					118	2060	0.572	84.30	1.619			
Wang and Zhang [120]	150	450	51.1					118	2060	0.572	88.65	1.446			
Watanabe et al. [28]	100	200	30.2	0.23	97.1 ^t	2589 ^t		73	3432	0.145	39.0	1.58	2.36	0.885	0.502 [^]
Watanabe et al. [28]	100	200	30.2	0.23	87.3 ^t	2707 ^t		73	3432	0.290	68.5	4.74 ^a	3.09	0.997	0.657
Watanabe et al. [28]	100	200	30.2	0.23	87.3 ^t	2667 ^t		73	3432	0.430	92.1	5.55 ^a	2.65	0.867	0.564
Wu et al. [31]	150	300	23.0		115 ^t	2324 ^t		120	2000	0.286	53.0				
Wu et al. [35]	150	300	23.1	0.267	115 ^t	2324 ^t		120	2000	0.286	45.2	2.31			
Wu et al. [35]	150	300	23.1	0.267	115 ^t	2324 ^t		120	2000	0.286	50.7	3.03 ^a			
Wu et al. [35]	150	300	23.1	0.267	115 ^t	2324 ^t		120	2000	0.286	53.7	3.29 ^a			
Wu et al. [122]	100	300	46.4	0.255				118	2060	0.286	78.26	0.903			
Wu et al. [122]	100	300	46.4	0.255				118	2060	0.572	128.49	1.879			

^t denotes FRP properties calculated based on total nominal ply thickness of fiber sheet

^f denotes fiber properties established to be inaccurate based on the analysis of the database

^a denotes inconsistent axial strain when compared with overall trend in the database

[^] denotes inconsistent k_{ε} values when compared with overall trend in the database

Table 6. Test database of HM and UHM CFRP-wrapped concrete specimens

Paper	Specimen Dimensions		Concrete Properties		FRP Properties			Fiber Properties			Measured Ultimate Conditions			Hoop Rupture Strain Reduction Factors	
	D (mm)	H (mm)	f'_{co} (MPa)	ε_{co} (%)	E_{frp} (GPa)	f_{frp} (MPa)	t_{frp} (mm)	E_f (GPa)	f_f (MPa)	t_f (mm)	f'_{cc} (MPa)	ε_{cu} (%)	$\varepsilon_{h,rupt}$ (%)	$k_{\varepsilon,frp}$	$k_{\varepsilon,f}$
Bullo [64]	150	300	32.54	0.248				390	3000	0.165	52.63	0.833	0.467		0.607
Bullo [64]	150	300	32.54	0.248				390	3000	0.165	56.59	0.928	0.52		0.676
Bullo [64]	150	300	32.54	0.248				390	3000	0.165	61.11	0.833	0.421		0.547
Bullo [64]	150	300	32.54	0.248				390	3000	0.495	97.33	1.817	0.639		0.831 [^]
Bullo [64]	150	300	32.54	0.248				390	3000	0.495	83.75	1.265	0.439		0.571
Bullo [64]	150	300	32.54	0.248				390	3000	0.495	100.16	1.687	0.539		0.701
Cui and Sheikh [68]	152	305	45.7	0.243				436	3314	0.16	67.5	1.11 ^a	0.789		1.038 [^]
Cui and Sheikh [68]	152	305	45.7	0.243				436	3314	0.16	64.1	1.03 ^a	0.769		1.012 [^]
Cui and Sheikh [68]	152	305	45.7	0.243				436	3314	0.33	84.2	1.33	0.642		0.845 [^]
Cui and Sheikh [68]	152	305	45.7	0.243				436	3314	0.33	83.1	1.23	0.634		0.834 [^]
Cui and Sheikh [68]	152	305	45.7	0.243				436	3314	0.49	99.7	1.56	0.603		0.793 [^]
Cui and Sheikh [68]	152	305	45.7	0.243				436	3314	0.49	94.9	1.43	0.546		0.718 [^]
Hosotani et al. [76]	200	600	41.7	0.34	439	3972	0.676	392	2943	0.652	90	1.5			
Lin and Chen [38]	120	240	32.7		157.54	770	0.5				51.0				
Lin and Chen [38]	120	240	32.7		157.54	770	0.5				49.6				
Lin and Chen [38]	120	240	32.7		157.54	770	1.0				77.3				
Lin and Chen [38]	120	240	32.7		157.54	770	1.0				68.9				
Rousakis [105]	150	300	25.2	0.311				377	4410	0.17	41.6	1.437	0.695		0.594
Rousakis [105]	150	300	25.2	0.311				377	4410	0.17	38.8	1.206	0.581		0.497
Rousakis [105]	150	300	25.2	0.311				377	4410	0.34	60.1	1.881	0.641		0.548
Rousakis [105]	150	300	25.2	0.311				377	4410	0.34	55.9	2.097	0.551		0.471
Rousakis [105]	150	300	25.2	0.311				377	4410	0.51	67.0	2.452	0.449		0.384
Rousakis [105]	150	300	25.2	0.311				377	4410	0.51	67.3	2.432	0.368		0.315
Rousakis [105]	150	300	47.4	0.308				377	4410	0.17	72.3	1.085	0.772		0.660
Rousakis [105]	150	300	47.4	0.308				377	4410	0.17	64.4	0.866	0.513		0.439
Rousakis [105]	150	300	47.4	0.308				377	4410	0.34	82.4	1.399	0.656		0.561
Rousakis [105]	150	300	47.4	0.308				377	4410	0.34	82.4	1.350	0.537		0.459
Rousakis [105]	150	300	47.4	0.308				377	4410	0.51	96.3	1.585	0.443		0.379
Rousakis [105]	150	300	47.4	0.308				377	4410	0.51	95.2	1.687	0.578		0.494
Rousakis [105]	150	300	51.8	0.298				377	4410	0.17	78.7	0.748	0.543		0.464
Rousakis [105]	150	300	51.8	0.298				377	4410	0.17	72.8	0.663	0.398		0.340
Rousakis [105]	150	300	51.8	0.298				377	4410	0.34	95.4	1.047	0.551		0.471
Rousakis [105]	150	300	51.8	0.298				377	4410	0.34	90.7	1.001	0.364		0.311
Rousakis [105]	150	300	51.8	0.298				377	4410	0.51	110.5	1.292	0.438		0.374
Rousakis [105]	150	300	51.8	0.298				377	4410	0.51	103.6	1.203	0.310		0.265 [^]
Rousakis [105]	150	300	51.8	0.298				377	4410	0.85	112.7	1.593	0.289		0.247 [^]
Rousakis [105]	150	300	51.8	0.298				377	4410	0.85	126.7	1.612	0.360		0.308
Matthys et al. [11]	150	300	34.9	0.21	480	1100		640	2650	0.235	41.3 ^s	0.40	0.19	0.829 [^]	0.459
Suter and Pinzelli [113]	150	300	44.7					640	2650	0.38	91.98	0.534			
Watanabe et al. [28]	100	200	30.2	0.23	628 ^t	1579 ^t		637	2452	0.14	41.7	0.57	0.23	0.916 [^]	0.598
Watanabe et al. [28]	100	200	30.2	0.23	629 ^t	1824 ^t		637	2452	0.28	56.0	0.88	0.22	0.759 [^]	0.572
Watanabe et al. [28]	100	200	30.2	0.23	576 ^t	1285 ^t		637	2452	0.42	63.3	1.30	0.22	0.987 [^]	0.572
Wu et al. [31]	150	300	23.0		563 ^t	2544 ^t		540	1900	0.286	50.0				
Wu et al. [35]	150	300	23.1	0.267	563 ^t	2544 ^t		540	1900	0.286	50.5	1.27 ^a			
Wu et al. [35]	150	300	23.1	0.267	563 ^t	2544 ^t		540	1900	0.286	48.9	1.20 ^a			

^t denotes FRP properties calculated based on total nominal ply thickness of fiber sheet

[^] denotes inconsistent k_{ε} values when compared with overall trend in the database

Table 8. Average hoop rupture strain reduction factor (k_e) with FRP type and confinement technique

Specimens	$k_{e,f}$			$k_{e,frp}$		
	Number	SD	Average	Number	SD	Average
All	201	0.135	0.675	150	0.125	0.709
All wrapped	186	0.134	0.675	146	0.126	0.707
CFRP wrapped	131	0.115	0.680	116	0.127	0.682
GFRP wrapped	25	0.084	0.793	23	0.059	0.803
AFRP wrapped	8	0.087	0.732	7	0.066	0.809
HM CFRP wrapped	22	0.115	0.493	-	-	-
UHM CFRP wrapped	-	-	-	-	-	-
All tube-encased	15	0.157	0.675	4	0.047	0.775
CFRP tube-encased	4	0.033	0.690	-	-	-
GFRP tube-encased	5	0.094	0.723	4	0.047	0.775
AFRP tube-encased	4	0.055	0.775	-	-	-
HM CFRP tube-encased	-	-	-	-	-	-
UHM CFRP tube-encased	2	0.051	0.326	-	-	-

Table 9. Variation of strength enhancement coefficients (k_1) with FRP type and confinement technique

Specimens	k_1		
	Number	R^2	Average
All	753	0.799	3.22
All wrapped	684	0.806	3.26
CFRP wrapped	426	0.870	3.67
GFRP wrapped	149	0.759	2.49
AFRP wrapped	67	0.889	3.30
HM CFRP wrapped	34	0.772	4.96
UHM CFRP wrapped	8	-	-
All tube-encased	69	0.759	2.94
CFRP tube-encased	14	0.907	2.87
GFRP tube-encased	36	0.731	2.92
AFRP tube-encased	12	0.811	2.95
HM CFRP tube-encased	3	-	-
UHM CFRP tube-encased	4	-	-

Table 10. Variation of strain enhancement coefficients (k_2) with FRP type and confinement technique

Specimens	All, k_2			AS, k_2			AFL, k_2			AML, k_2		
	Number	R^2	Average	Number	R^2	Average	Number	R^2	Average	Number	R^2	Average
All	511	0.786	0.271	53	0.583	0.270	179	0.831	0.297	215	0.723	0.261
All wrapped	462	0.753	0.266	50	0.564	0.271	134	0.809	0.296	215	0.723	0.261
CFRP wrapped	282	0.682	0.267	48	0.546	0.269	53	0.677	0.296	143	0.660	0.259
GFRP wrapped	109	0.820	0.262	-	-	-	40	0.719	0.281	60	0.765	0.249
AFRP wrapped	36	0.613	0.265	2	1.000	0.339	15	0.847	0.355	8	0.981	0.334
HM CFRP wrapped	30	0.688	0.320	-	-	-	26	0.714	0.320	4	0.863	0.321
UHM CFRP wrapped	5	-	-	-	-	-	-	-	-	-	-	-
All tube-encased	49	0.883	0.298	3	0.433	0.258	45	0.870	0.299	-	-	-
CFRP tube-encased	12	0.959	0.268	-	-	-	11	0.965	0.272	-	-	-
GFRP tube-encased	22	0.862	0.298	3	0.433	0.258	19	0.797	0.300	-	-	-
AFRP tube-encased	12	0.351	0.302	-	-	-	12	0.351	0.302	-	-	-
HM CFRP tube-encased	3	-	-	-	-	-	3	-	-	-	-	-
UHM CFRP tube-encased	-	-	-	-	-	-	-	-	-	-	-	-

Specimen instrumentation notes:

AS denotes axial strains were determined from axial strain gauges mounted on the surface of the specimens at mid-height of specimens

AFL denotes axial strains were determined from LVDTs or dial gauges mounted on loading platens to measure deformations along the full height of specimens

AML denotes axial strains were determined from LVDTs or dial gauges mounted on the specimens to measure deformations within a gauge length along the height of specimens

Table 11. Statistics of strength enhancement ratio (f'_{cc}/f'_{co}) predictions of best performing models

Model	Prediction of f'_{cc}/f'_{co}			
	Test data	Average Absolute Error (%)	Mean (%)	Standard Deviation (%)
Proposed model	753	11.2	99.6	13.7
Teng et al. [48]	753	11.8	98.8	14.5
Lam and Teng [3]	753	12.4	99.4	15.3
Wu and Zhou [52]	753	12.4	102.1	15.5
Wu and Wang [51]	753	12.7	101.4	15.7
Wei and Wu [50]	753	12.7	101.5	15.7
Al-Salloum and Siddiqui [45]	753	12.7	101.7	15.8
Realfonzo and Napoli [7]	753	12.7	103.2	15.8
Bisby et al. [5]	753	12.8	101.9	15.8
Jiang and Teng [32]	753	12.9	93.9	14.6

Table 12. Statistics of strain enhancement ratio ($\epsilon_{cu}/\epsilon_{co}$) predictions of best performing models

Model	Prediction of $\epsilon_{cu}/\epsilon_{co}$			
	Test data	Average Absolute Error (%)	Mean (%)	Standard Deviation (%)
Proposed model	511	21.7	100.5	27.2
Tamuzs et al. [43]	511	26.3	108.4	35.0
Wei and Wu [50]	511	28.7	98.0	35.8
Binici [46]	511	29.2	92.3	34.8
Jiang and Teng [42]	511	29.5	116.1	38.5
Youssef et al. [53]	511	30.0	112.5	39.0
Teng et al. [49]	511	30.2	117.6	39.0
Fahmy and Wu [47]	511	30.5	99.5	38.9
Teng et al. [48]	511	30.5	117.0	39.3
De Lorenzis and Tepfers [4]	511	31.3	77.9	27.9

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Figure 7. Average absolute error in model predictions of: (a) strength enhancement ratios (f'_{cc}/f'_{co}), (b) strain enhancement ratios ($\epsilon_{cu}/\epsilon_{co}$)

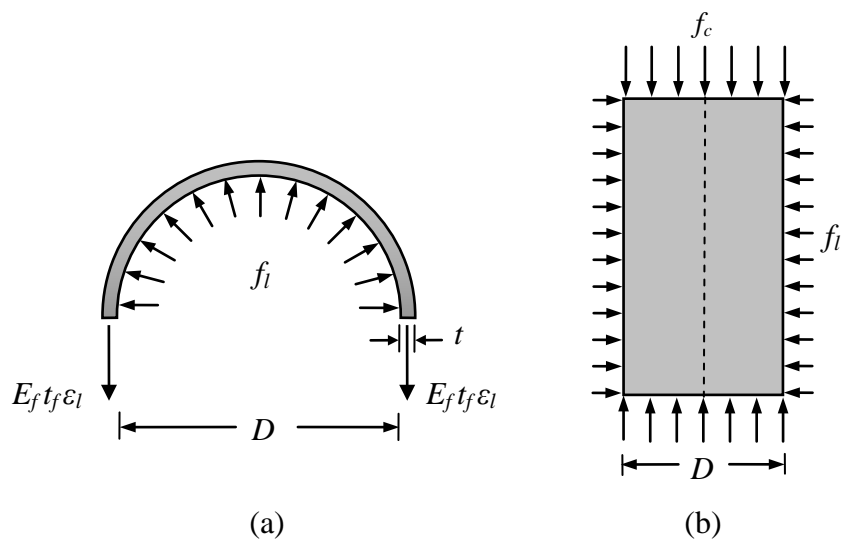


Figure 1. Confining action of FRP shell on concrete core: (a) FRP shell; (b) Concrete core

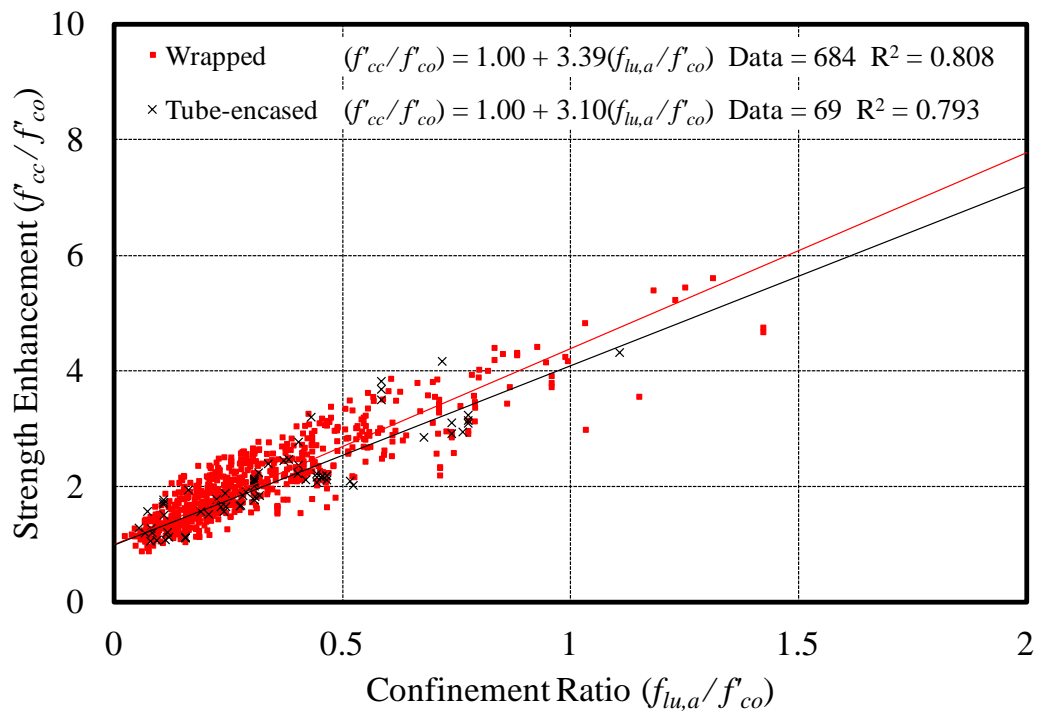


Figure 2. Variation of strength enhancement ratio with confinement ratio

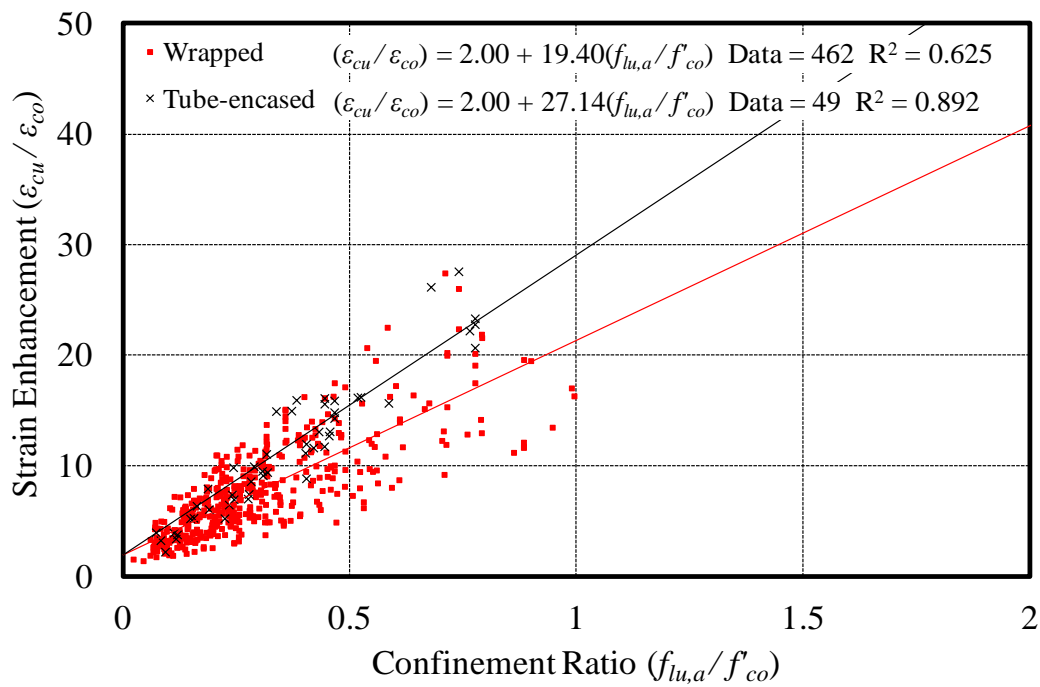
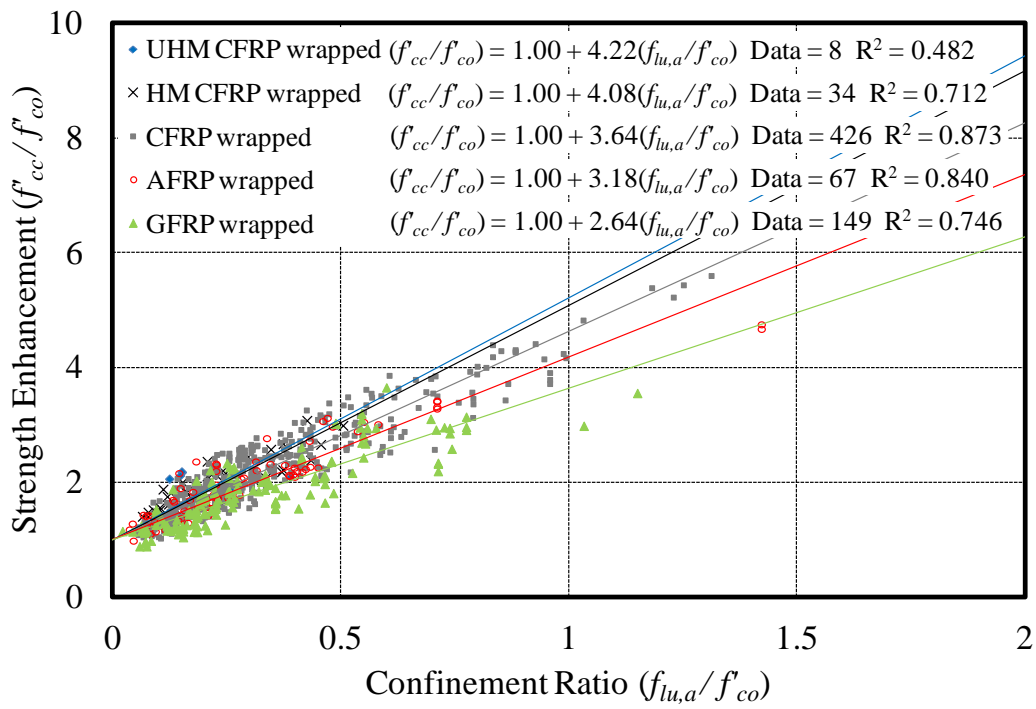
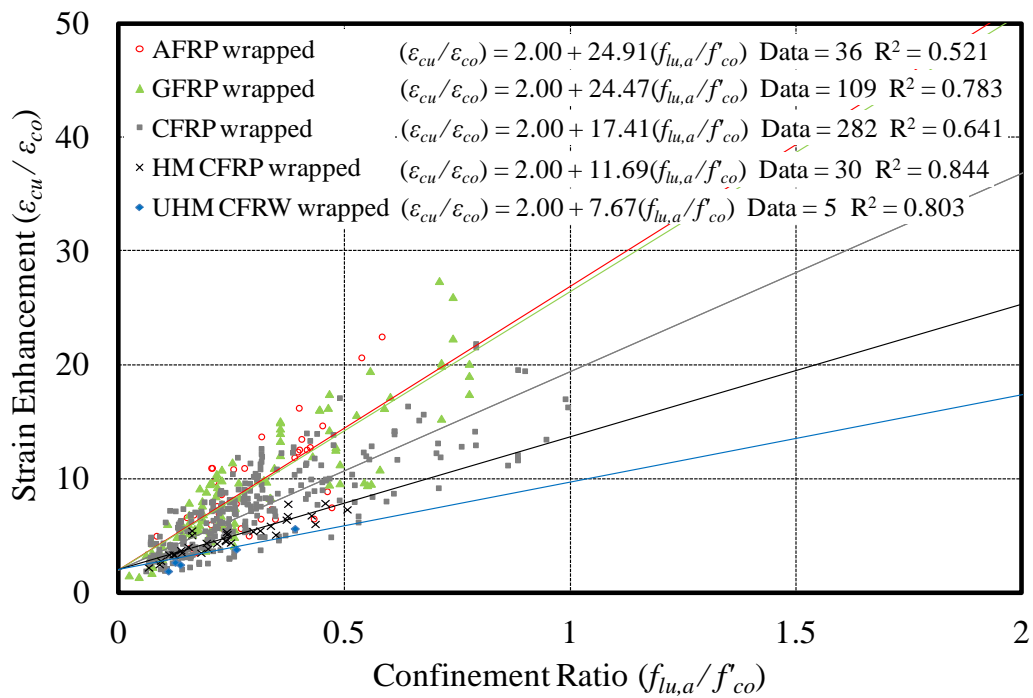


Figure 3. Variation of strain enhancement ratio with confinement ratio



(a)



(b)

Figure 4. Influence of FRP type on ultimate conditions of FRP-confined concrete: (a) compressive strength; (b) ultimate axial strain

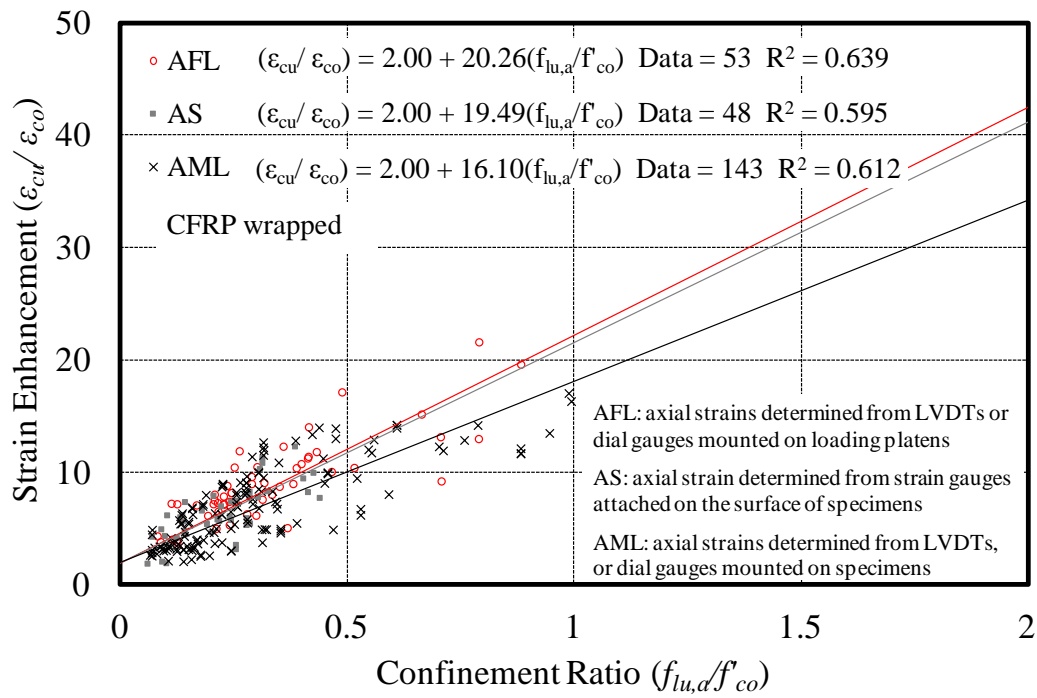
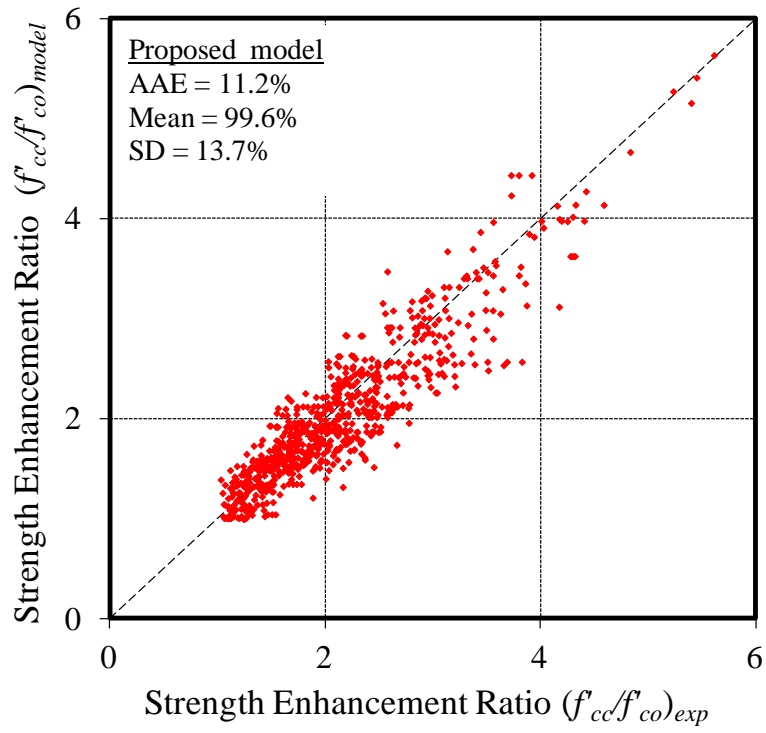
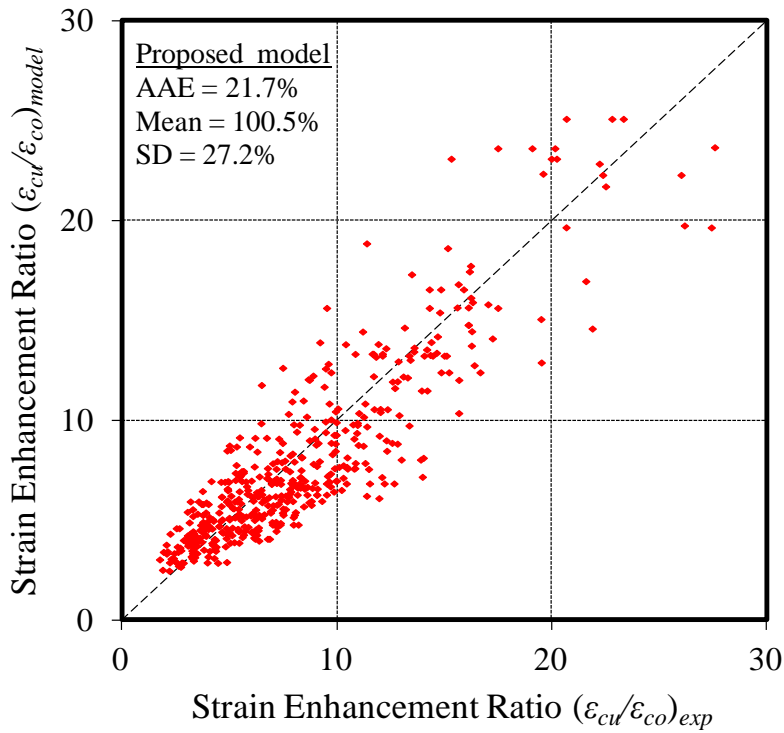


Figure 5. Influence of measurement method on ultimate strain of CFRP-wrapped concrete

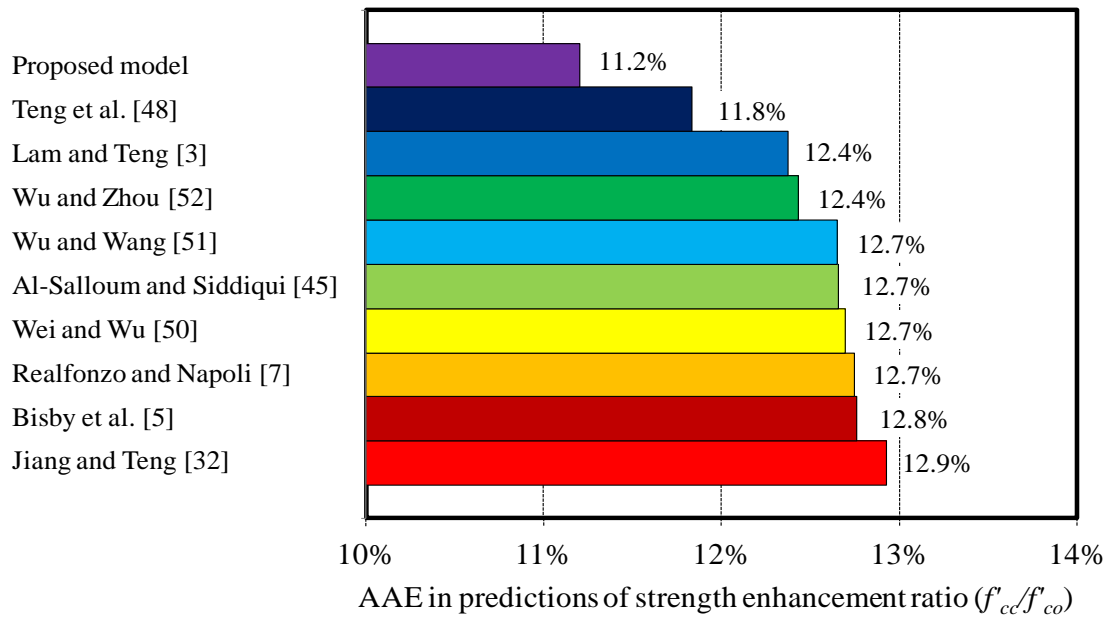


(a)

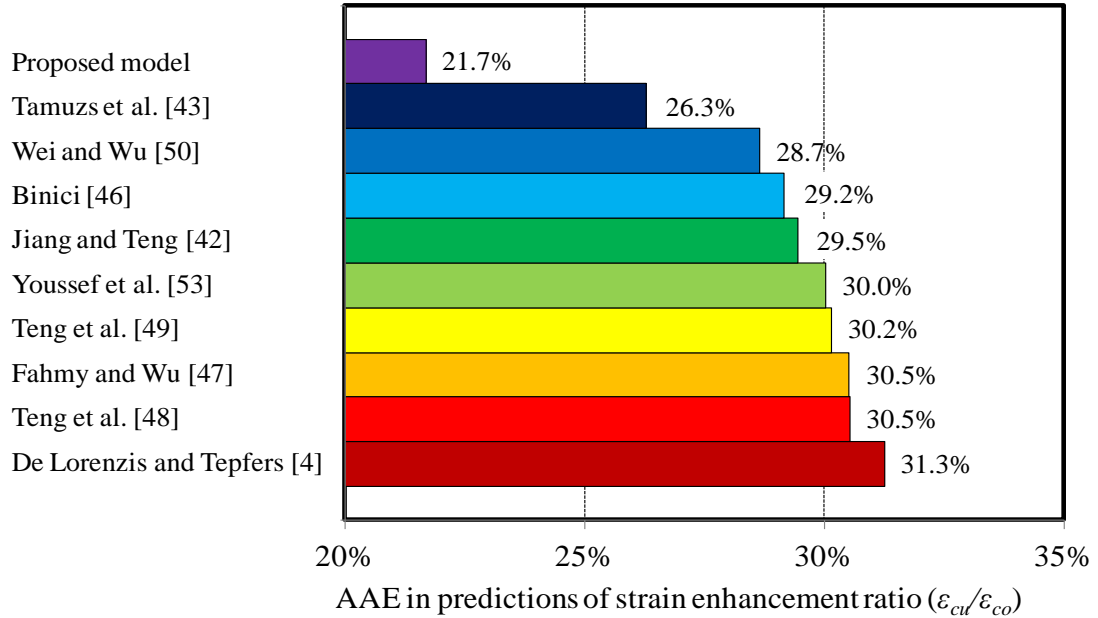


(b)

Figure 6. Comparison of model predictions of: (a) strength enhancement ratios (f'_{cc}/f'_{co}) and (b) strain enhancement ratios $(\epsilon_{cu}/\epsilon_{co})$ with experimental data



(a)



(b)

Figure 7. Average absolute error in model predictions of: (a) strength enhancement ratios (f'_{cc}/f'_{co}),

(b) strain enhancement ratios ($\epsilon_{cu}/\epsilon_{co}$)